

LEE COUNTY DEPARTMENT OF TRANSPORTATION

ABLE CANAL PATHWAY

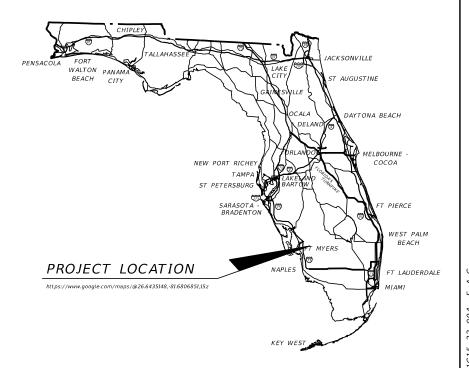
FINANCIAL PROJECT ID 435351-2-58-01
ETDM No: 14223
STRUCTURES PLANS

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415-001	BAR BENDING DETAILS (STEEL)
450-450	FLORIDA SLAB BEAM TYPICAL DETAILS AND NOTES
450-452	15" FLORIDA SLAB BEAM
455-001	SQUARE PRESTRESSED CONCRETE PILES - TYPICAL DETAILS AND NOTES
455-002	SQUARE PRESTRESSED CONCRETE PILE SPLICES
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458-110	EXPANSION JOINT SYSTEM - POURED JOINT WITH BACKER ROD
515-051	BRIDGE PEDESTRIAN/BICYCLE RAILING (STEEL)
515-052	PEDESTRIAN/BICYCLE RAILING (STEEL)



STRUCTURES PLANS ENGINEER OF RECORD:

LUCIO MARTINEZ, P.E.
P.E. No.: 82380
KISINGER CAMPO AND ASSOCIATES CORP.
13461 PARKER COMMONS BLVD., STE. 104
FORT MYERS, FLORIDA 33912
(239) 278-5999
VENDOR No. 59-1677145

NOTE: THE SCALE OF THESE PLANS MAY HAVE CHANGED DUE TO REPRODUCTION

LEE COUNTY PROJECT MANAGER:

RYAN HYLEN

FISCAL YEAR	SHEET NO.	
25	B-1	

CE N STATE OF OR I DAG

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ON THE DATE ADJACENT TO THE SEAL

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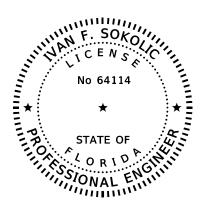
Kisinger Campo & Associates Corp. 13461 Parker Commons Blvd., Ste. 104 Fort Myers, FL 33912 Lucio Martinez, PE No. 82380

THE ABOVE NAMED PROFESSIONAL ENGINEER SHALL BE RESPONSIBLE FOR THE FOLLOWING SHEETS IN ACCORDANCE WITH RULE 61G15-23.004, F.A.C.

STRUCTURE PLANS

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B-31	GRAVITY WALL



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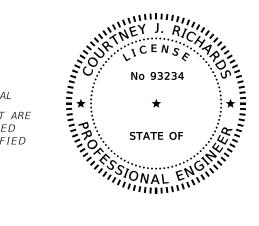
Ardaman And Associates, Inc. 9970 Bavarian Road Fort Myers, FL 33913 Ivan F. Sokolic, PE No. 64114

THE ABOVE NAMED PROFESSIONAL ENGINEER SHALL BE RESPONSIBLE FOR THE FOLLOWING SHEETS IN ACCORDANCE WITH RULE 61G15-23.004, F.A.C.

STRUCTURE PLANS

SHEET NO.	SHEET DESCRIPTION
B-2	SIGNATURE SHEET
B-7	REPORT OF CORE BORINGS

THIS ITEM HAS BEEN DIGITALLY



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Kisinger Campo & Associates Corp. 13461 Parker Commons Blvd., Fort Myers, FL 33912 Courtney J. Richards, PE No. 93234

THE ABOVE NAMED PROFESSIONAL ENGINEER SHALL BE RESPONSIBLE FOR THE FOLLOWING SHEETS IN ACCORDANCE WITH RULE 61G15-23.004, F.A.C.

STRUCTURE PLANS

SHEET NO.	SHEET DESCRIPTION
B-2	SIGNATURE SHEET
B-6	BRIDGE HYDRAULIC RECOMMENDATIONS

ABLE CANAL PATHWAY OVER CHARLIE DIVERSION CANAL

REVISIONS DESCRIPTION DESCRIPTION DATE



13461 Parker Commons Blvd., Ste. 104 Fort Myers, Florida 33912

PROJECT ABLE CANAL PATHWAY

LEE COUNTY DEPARTMENT OF TRANSPORTATION FINANCIAL PROJECT ID

SIGNATURE SHEET

SHEET NO.

B-2

435351-2-58-01

CECTION	PAY ITEM	DAY ITEM DECORIDITION	LOCATION		QUANT ITY		TO	ΓAL	DESIGN	CONSTRUCTION
SECT ION	NO.	PAY ITEM DESCRIPTION	LOCATION	UNIT	Р	F	Р	F	NOTES	REMARKS
LUMP SUM ITEMS	108 - 1	MONITOR EXISTING STRUCTURES - INSPECTION AND SETTLEMENT MONITORING	BRIDGE	EA	1		1			
LUMF SUM TIEMS	108 - 2	MONITOR EXISTING STRUCTURES - VIBRATION MONITORING	BR I DGE	EA	1		1			
	455 - 34 - 3	PRESTRESSED CONCRETE PILING, 18" SQ	END BENT 1	LF	119		753			
			INT. BENT 2		115					
			INT. BENT 3		173					
			INT. BENT 4		115					
FOUNDAT I ON			INT. BENT 5		172					
			END BENT 6		59					
	455 - 143 - 3	TEST PILES - PRESTRESSED CONCRETE, 18" SQ	INT BENT. 2	LF	75		224			
			INT BENT. 4		75					
			END BENT 6		74					
	400 - 4 - 5	CLASS IV CONCRETE, SUBSTRUCTURE	END BENT 1	CY	6.9		31.4			
			INT. BENT 2		4.4					
			INT. BENT 3		4.4					
			INT. BENT 4		4.4					
			INT. BENT 5		4.4					
SUBSTRUCTURE			END BENT 6		6.9					
30237710070712	415-1-5	REINFORCING STEEL - BRIDGE SUBSTRUCTURE	END BENT 1	LB	849		3098			
			INT. BENT 2		350					
			INT. BENT 3		350					
			INT. BENT 4		350					
			INT. BENT 5 END BENT 6		350 849					
	400 - 2 - 10	CLASS II CONCRETE, APPROACH SLABS	APPROACH SLAB 1	CY	5.4		10.8			
	400-2-10	CLASS II CONCRETE, APPROACH SLADS	APPROACH SLAB 2	C1	5.4		10.0			
APPROACH SLABS	115-1-9	REINFORCING STEEL - APPROACH SLABS	APPROACH SLAB 1	LB	989		1978			
ALL NOACH SLADS	713-1-9	ALTINI ONCING SIELL - AFFROACH SEADS	APPROACH SLAB 2	LD	989		1370			

ABLE CANAL PATHWAY OVER CHARLIE DIVERSION CANAL

	REV I			
DATE	DESCRIPTION	DATE	DESCRIPTION	
				13461 Parker Commons Blvd, Suite 10 Fort Myers, Florida 33912 KISINGER CAMPO Engineer of Record: Lucio Martinez & ASSOCIATES P.E. No.: 82380

PROJECT

LEE COUNTY DEPARTMENT OF TRANSPORTATION FINANCIAL PROJECT ID ABLE CANAL PATHWAY 435351-2-58-01

SUMMARY OF STRUCTURES QUANTITIES (1 OF 2)

SHEET NO. BQ-1

CECTION	PAY ITEM	DAY ITEM DECERIPTION	LOCATION		QUAN	TITY	TOTAL	DESIG	CONSTRUCTION
SECTION	NO .	PAY ITEM DESCRIPTION	LOCATION	UNIT	Р	F	P F		
	400 - 2 - 47	CONCRETE CLASS II, CAST-IN-PLACE TOPPING WITH SHRINKAGE REDUCING ADMIXTURE	SPAN 1	CY	21.8		108.7		
			SPAN 2		21.7				
			SPAN 3		21.7				
			SPAN 4		21.7				
			SPAN 5		21.8				
	400 - 148	PLAIN NEOPRENE BEARING PADS	SPAN 1	CF	1.2		6.0		
			SPAN 2		1.2				
			SPAN 3		1.2				
			SPAN 4		1.2				
			SPAN 5		1.2				
	415-1-4	REINFORCING STEEL - BRIDGE SUPERSTRUCTURE	SPAN 1	LB	4418		22090		
			SPAN 2		4418				
			SPAN 3		4418				
SUPERSTRUCTURE			SPAN 4		4418				
			SPAN 5		4418				
	450 - 8 - 22	PRESTRESSED BEAM: FLORIDA SLAB BEAM, BEAM DEPTH 15" CARBON STEEL, WIDTH 52-54"	SPAN 1	LF	156		780		
			SPAN 2		156				
			SPAN 3		156				
			SPAN 4		156				
			SPAN 5		156				
	458 - 1 - 11	BRIDGE DECK EXPANSION JOINT, NEW CONSTRUCTION, F&I POURED JOINT WITH BACKER ROD	END BENT 1	LF	14		84		
			INT. BENT 2		14				
			INT. BENT 3		14				
			INT. BENT 4		14				
			INT. BENT 5		14				
			END BENT 6		14				
	515-2-211	PEDESTRIAN / BICYLCE RAILING, STEEL, 42" TYPE 1	APPROACH SLAB 1	LF	10		551		
BARRIERS			BR I DGE		531				
			APPROACH SLAB 2		10				
	530 - 1 - 100	RIPRAP, SAND-CEMENT BAGS	END BENT 1	CY	3.5		7.0		
			END BENT 6		3.5				
	530-3-3	RIPRAP - RUBBLE, BANK AND SHORE	END BENT 1	TN	230.1		452.6		
SLOPE			END BENT 6		222.5				
PROTECTION	530-74	BEDDING STONE	END BENT 1	TN	81.7		160.7		
			END BENT 6		79.0				
	LC01	FLOATING BOOM BARRIER	CANAL	LF	300.0		300		

	SUMMARY OF WALLS												
WALL	PAY ITEM	PAY ITEM DESCRIPTION	LOCATION	SIDE	UNIT	QUAN	TITY	TO	ΓΑL	DESIGN	CONSTRUCTION		
NO.	NO .	TAI THEM BESCHITTION	STA. TO STA.	JIDL		JIDL	0,41,	P	F	P	F	NOTES	REMARKS
GW - 1	400 - 0 - 11	CONCRETE CLASS NS, GRAVITY WALL	445+68.00 TO 447+09.00	LT.	CY	26.9		26.9					

ABLE CANAL PATHWAY OVER CHARLIE DIVERSION CANAL

I		REVISIONS						
DEPA]	DESCRIPTION	DATE	DESCRIPTION	DATE			
1013111	13461 Parker Commons Blvd, Suite 104							
PRO.	Fort Myers, Florida 33912							
ABLE CANA	KISINGER CAMPO Engineer of Record: Lucio Martinez & ASSOCIATES P.E. No.: 82380							

4	LEE CO DEPARTMENT OF	
	PROJECT	FINANCIAL PROJECT ID
	ABLE CANAL PATHWAY	435351-2-58-01

SUMMARY OF STRUCTURES QUANTITIES (2 OF 2) SHEET NO. BQ-2

- 1. FDOT STRUCTURES MANUAL DATED JANUARY 2024 AND SUBSEQUENT STRUCTURES DESIGN BULLETINS.
- 2. AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS (AASHTO) LOAD AND RESISTANCE FACTOR DESIGN (LRFD) BRIDGE DESIGN SPECIFICATIONS, 9TH EDITION.
- 3. AMERICAN ASSOCIATION OF STATE HIGHWAY AND TRANSPORTATION OFFICIALS (AASHTO) LOAD AND RESISTANCE FACTOR DESIGN (LRFD) GUIDE SPECIFICATIONS FOR THE DESIGN OF PEDESTRIAN BRIDGES 2009 WITH 2015 INTERIMS.
- 4. FDOT DESIGN MANUAL DATED JANUARY 2024 AND SUBSEQUENT ROADWAY DESIGN BULLETINS.

GOVERNING STANDARDS AND CONSTRUCTION SPECIFICATIONS

FLORIDA DEPARTMENT OF TRANSPORTATION, FY 2024-25 STANDARD PLANS AND REVISED INDEX DRAWINGS AS APPENDED HEREIN, AND FY 2024-25 STANDARD SPECIFICATIONS FOR ROAD AND BRIDGE CONSTRUCTION, AS AMENDED BY CONTRACT DOCUMENTS.

VERTICAL DATUM

ELEVATIONS ARE ACCORDING TO NORTH AMERICAN VERTICAL DATUM OF 1988 (NAVD 88).

ENVIRONMENT

ENVIRONMENTAL CLASSIFICATION				
SUPERSTRUCTURE	SUBSTRUCTURE (CONCRETE)			
SLIGHTLY AGGRESSIVE	MODERATELY AGGRESSIVE (*)			

^{*} RESISTIVITY = 1,848 OHM-CM

DESIGN METHODOLOGY

- 1. LOAD AND RESISTANCE FACTOR DESIGN (LRFD) METHOD USING STRENGTH, SERVICE, AND FATIGUE LIMIT STATES.
- 2. REDUNDANCY FACTORS
 - A. END BENT PILES $\eta_{\scriptscriptstyle R}=1.10$
- 3. OPERATIONAL IMPORTANCE FACTOR A. ALL BRIDGES $\eta_c = 1.00$

DESIGN LOADINGS

1. LIVE LOADS:

VEHICLE LOAD: H10 WITHOUT DYNAMIC LOAD ALLOWANCE PEDESTRIAN LOAD: 90 PSF

2. DEAD LOADS:

PEDESTRIAN RAILING = 30 PIF REINFORCED CONCRETE = 150 PCF

DESIGN DOES NOT INCLUDE AN ALLOWANCE OF 15 PSF FOR FUTURE WEARING SURFACE.

3. WIND LOADS:

Imartinez

WIND LOADS IN ACCORDANCE WITH CHAPTER 2.4 OF THE FDOT STRUCTURES MANUAL, VOLUME 1 STRUCTURES DESIGN GUIDELINES

4. CONSTRUCTION LOADS:

FINISH MACHINE LOAD: 7.0 KIPS

FINISHING MACHINE WHEEL LOCATION BEYOND THE EDGE OF DECK OVERHANG: 6 INCHES

CONSTRUCTION LIVE LOAD: 20 PSF EXTENDED OVER THE ENTIRE BRIDGE WIDTH AND 50-FEET IN LONGITUDINAL LENGTH CENTERED ON THE FINISHING MACHINE

LIVE LOAD AT OR NEAR THE OUTSIDE EDGE OF DECK DURING DECK CASTING: 75 PLF APPLIED AS A MOVING LOAD OVER A LENGTH OF

CONSTRUCTION INACTIVE DESIGN WIND SPEED: 90 MPH VELOCITY PRESSURE EXPOSURE COEFFICIENT (Kz): 1.00 CONSTRUCTION ACTIVE DESIGN WIND SPEED: 30 MPH

5. THERMAL DATA:

	MEAN	RISE FROM MEAN	FALL FROM MEAN	RANGE
CONCRETE	70 °F	35 °F	35 °F	70 °F

NO ALLOWANCE FOR UTILITY LOAD HAS BEEN INCLUDED IN THE DESIGN.

MATERIALS

- 1. REINFORCING STEEL: GRADE 60 CARBON STEEL PER SPECIFICATION SECTION 931.
- 2. CONCRETE:

CONCRETE CLASS	MINIMUM 28-DAY COMPRESSIVE STRENGTH (PSI)	LOCATION IN STRUCTURE
II (BRIDGE DECK)	BRIDGE DECKS* AND APPROACH SLABS	
IV	5500	C.I.P. SUBSTRUCTURE
V (SPECIAL)	6000	PRESTRESSED CONCRETE PILES
VI 8500		FLORIDA SLAB BEAMS

* C.I.P. TOPPING WITH SHRINKAGE REDUCING ADMIXTURE: THOROUGHLY SATURATE THE TOP SURFACE OF THE FSBs WITH WATER IN ACCORDANCE WITH SPECIFICATION 400 FOR 12 HOURS IMMEDIATELY PRIOR TO PLACING THE C-I-P TOPPING. REMOVE STANDING WATER PRIOR TO PLACING THE C-I-P TOPPING. CURE THE C-I-P TOPPING IN ACCORDANCE WITH THE SPECIFICATION 400 REQUIREMENTS FOR BRIDGE DECKS.

3. CONCRETE COVER:

CAST-IN-PLACE SUPERSTRUCTURE	2"
FLORIDA SLAB BEAM	**
C.I.P. SUBSTRUCTURE (CAST AGAINST EARTH)	4"
C.I.P. SUBSTRUCTURE (FORMED SURFACES)	3"
C.I.P. SUBSTRUCTURE (CHEEKWALLS)	2"

** COVER REQUIREMENTS SHALL BE IN ACCORDANCE WITH STANDARD PLANS INDEX 450-452.

CONCRETE COVER DIMENSIONS SHOWN IN THE PLANS DO NOT INCLUDE PLACEMENT AND FABRICATION TOLERANCES UNLESS SHOWN AS "MINIMUM COVER". SEE SPECIFICATIONS SECTION 415 FOR ALLOWABLE TOLERANCES. ALL DIMENSIONS PERTAINING TO THE LOCATION OF REINFORCING STEEL ARE TO CENTERLINE OF BAR EXCEPT WHERE CLEAR DIMENSIONS NOTED TO FACE OF CONCRETE.

CONCRETE SURFACE FINISH

A CLASS 2 FINISH COATING SHALL BE APPLIED TO THE PORTIONS OF THE STRUCTURES SHOWN ON THE SURFACE FINISH DETAILS.

ALL DIMENSIONS IN THESE PLANS ARE MEASURED IN FEET AND INCHES, EITHER HORIZONTALLY OR VERTICALLY, UNLESS OTHERWISE NOTED.

FOR DISPOSITION OF UTILITIES, SEE THE UTILITY ADJUSTMENT SHEETS IN THE ROADWAY PLANS.

CANAL MAINTENANCE

CONTRACTOR IS RESPONSIBLE FOR COORDINATION WITH CANAL MAINTENANCE AGENCY THROUGHOUT THE CONSTRUCTION PROCESS. LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT CONTACT:

MIKE COOK MCOOK@LA-MSID.COM 239-368-0044 EXT.16

SCREEDING DECKS

SCREED THE RIDING SURFACE OF THE BRIDGE DECK AND APPROACH SLABS TO ACHIEVE THE FINISH GRADE ELEVATION SHOWN IN THE PLANS. ACCOUNT FOR THEORETICAL DEFLECTIONS DUE TO SELF WEIGHT, DECK CASTING SEQUENCE, DECK FORMING SYSTEMS, CONSTRUCTION LOADS, AND TEMPORARY SHORING, ETC. AS REQUIRED.

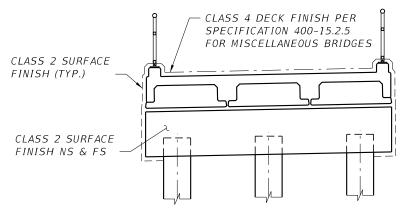
CONCRETE DETAILS

PROVIDE 3/4" CHAMFERS ON ALL EXPOSED EDGES EXCEPT AS OTHERWISE NOTED. CONSTRUCTION JOINTS WILL BE PERMITTED ONLY AT LOCATIONS INDICATED ON THE PLANS. ADDITIONAL CONSTRUCTION JOINTS OR ALTERATIONS TO THOSE SHOWN SHALL REQUIRE APPROVAL OF THE ENGINEER.

PAY ITEM NOTES

- 1. PAYMENT FOR INCIDENTAL ITEMS NOT SPECIFICALLY COVERED IN THE INDIVIDUAL BID ITEMS SHALL BE INCLUDED IN THE CONTRACT UNIT PRICE FOR THE BID ITEMS.
- 2. FIELD REVIEW THE PROJECT PRIOR TO PREPARING THE BID. LEE COUNTY DOES NOT ASSUME ANY LIABILITIES THAT MAY ARISE FROM FAILURE TO PROPERLY REVIEW THE PROJECT SITE PRIOR TO PREPARING THE BID.
- 3. COST OF FILL AND EXCAVATION FOR THE INSTALLATION OF SLOPE PROTECTION IS INCLUDED IN THE COST OF PAY ITEM 530-3-3.
- 4. INCLUDE THE COST OF REBAR DRIVEN THOUGH SAND-CEMENT RIPRAP IN THE CONTRACT UNIT PRICE FOR PAY ITEM NUMBER 530-1-100.
- 5. PAY ITEM LC01 INCLUDES FURNISHING AND INSTALLING THE FLOATING BOOM BARRIER. FLOATING BOOM BARRIER IS TO BE ORANGE WITH A HANGING 24 INCH DEBRIS SCREEN. CENTER BOOMS MUST BE LABELED "DANGER KEEP OUT". PROVIDE A PRODUCT EQUIVALENT TO THE EXISTING FLOATING BOOM BARRIERS UTILIZED BY THE LEHIGH ACRES MUNICIPAL SERVICES IMPROVEMENT DISTRICT. SUBMIT DETAILS OF PROPOSED PRODUCT FOR APPROVAL PRIOR TO ORDERING OF MATERIALS.

SURFACE FINISH DETAILS



FAA NOTICE OF PROPOSED CONSTRUCTION

PURSUANT TO CODE OF FEDERAL REGULATIONS (CFR), TITLE 14 PART 77.7 AND 77.9 FILE A NOTICE OF PROPOSED CONSTRUCTION WITH THE FEDERAL AVIATION ADMINISTRATION (FAA) AT LEAST 45 DAYS PRIOR TO CONSTRUCTION. SEE CFR TITLE 14 PART 77.7 AND 77.9 FOR FULL REQUIREMENTS.

ABBREVIATIONS

BFBWBACK FACE BACKWALL CLR. CLEAR

ΕF EACH FACE ES EACH SIDE FF FAR FACE

FFRW FRONT FACE BACKWALL FFRWFRONT FACE RETAINING WALL

ΝF NEAR FACE UNO UNLESS NOTED OTHERWISE

ABLE CANAL PATHWAY OVER CHARLIE DIVERSION CANAL

REVISIONS DATE DESCRIPTION DATE DESCRIPTION 13461 Parker Commons Blvd, Suite 104 Fort Myers, Florida 33912 Engineer of Record: Lucio Martinez & ASSOCIATES P.E. No.: 82380

LEE COUNTY DEPARTMENT OF TRANSPORTATION FINANCIAL PROJECT ID PROJECT

ABLE CANAL PATHWAY

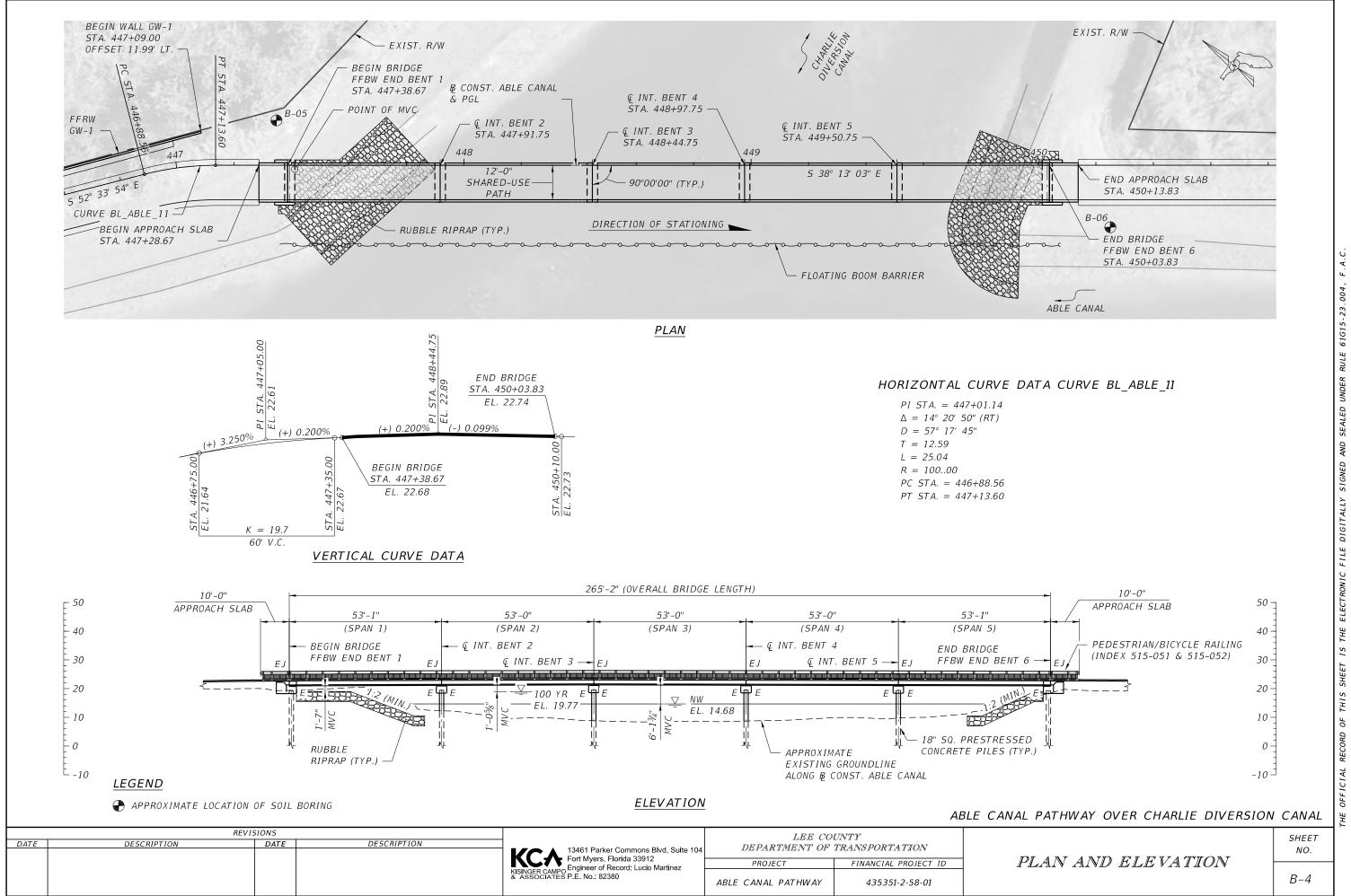
GENERAL NOTES

SHEET NO.

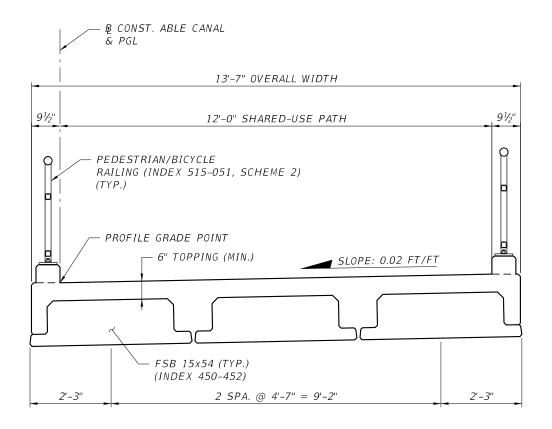
B-3

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435351-2-58-01







TYPICAL SECTION
(SPANS 1 THRU 5)

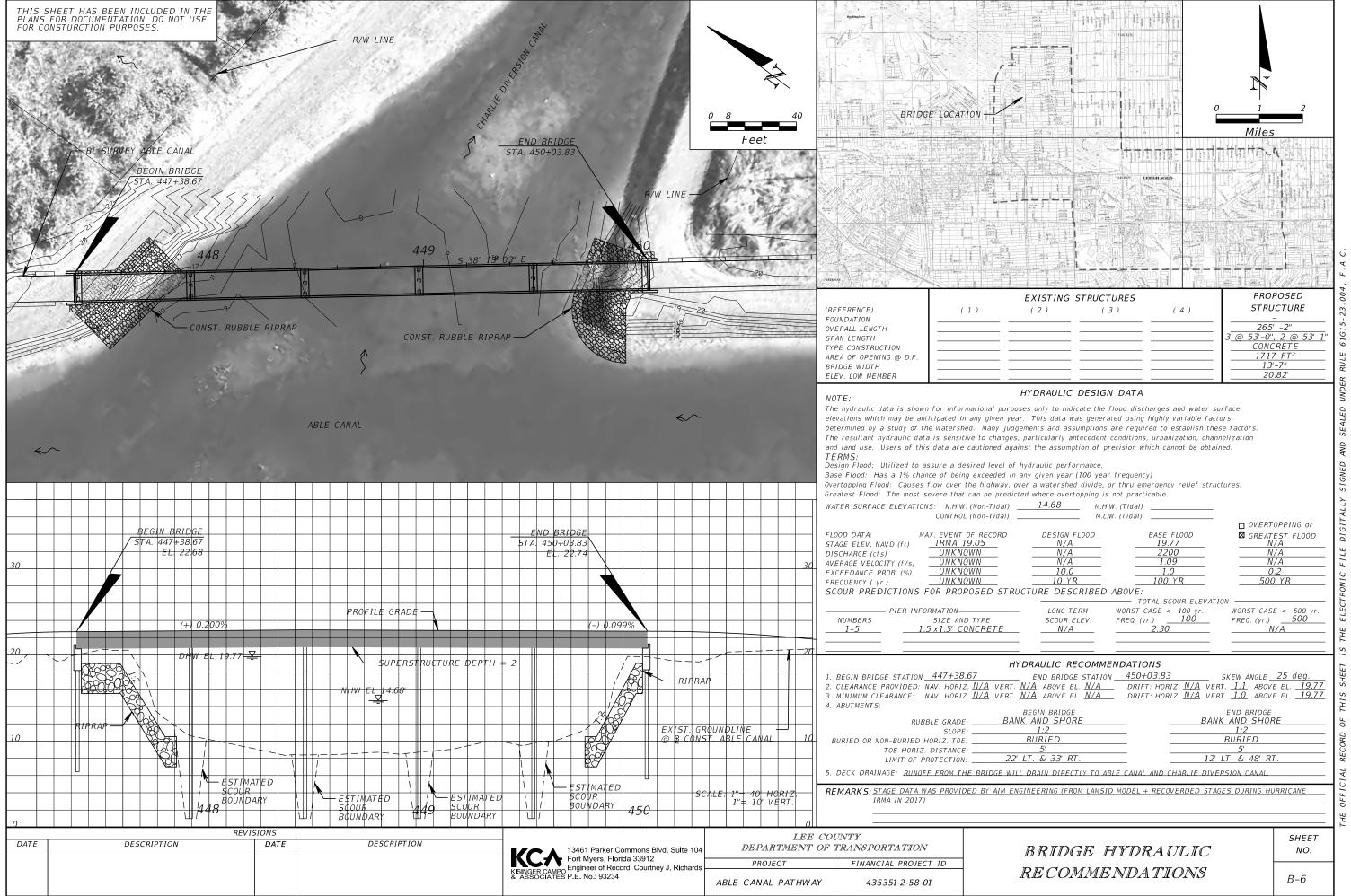
ABLE CANAL PATHWAY OVER CHARLIE DIVERSION CANAL

		REVISIONS				
DEPARTA]	DESCRIPTION	DATE	DESCRIPTION	DATE	
4 223 234 134	13461 Parker Commons Blvd, Suite 104					
PROJECT	Fort Myers, Florida 33912					
ABLE CANAL PA	KISINGER CAMPO Engineer of Record: Lucio Martinez & ASSOCIATES P.E. No.: 82380					

LEE COUNTY DEPARTMENT OF TRANSPORTATION					
	PROJECT	FINANCIAL PROJECT ID			
	ARIF CANAL PATHWAY	435351-2-58-01			

TYPICAL SECTION

SHEET NO. B-5



UPON COMPLETION OF EACH SPT BORING, THE BOREHOLE WAS GROUTED WITH CEMENT-BENTONITE SLURRY.

ALL SPT BORINGS WERE PERFORMED USING AN AUTOMATIC HAMMER

TO THE BORING TERMINATION DEPTH
GEOGRAPHICAL GPS COORDINATE LOCATIONS WERE DETERMINED
USING A 4 SATELLITE MINIMUM AUTONOMOUS SOLUTION FROM A WAAS HAND HELD GPS UNIT.

THE BORING LOGS SHOWN REPRESENT SUBSURFACE CONDITIONS
WITHIN THE BOREHOLE AT THE TIME OF DRILLING.
NO WARRANTY AS TO THE SUBSURFACE CONDITION, STRATA DEPTH
OF SOIL CONSISTENCY BETWEEN OR OUTSIDE BORING LOCATIONS IS
EXPRESSED OR IMPLIED BY THIS DRAWING.

WHILE THE BORINGS ARE REPRESENTATIVE OF SUBSURFACE CONDITIONS AT THEIR RESPECTIVE LOCATIONS AND FOR THEIR RESPECTIVE VERTICAL REACHES, LOCAL VARIATIONS CHARACTERISTIC OF THE SUBSURFACE MATERIALS OF THE REGION ARE ANTICIPATED AND MAY BE ENCOUNTERED.

THE BORING LOGS AND RELATED INFORMATION ARE BASED ON THE DRILLER'S LOGS AND VISUAL EXAMINATION OF SELECTED SAMPLES IN THE LABORATORY. THE DELINEATION BETWEEN SOIL TYPES SHOWN ON THE LOGS IS APPROXIMATE AND THE DESCRIPTION REPRESENTS OUR INTERPRETATION OF SUBSURFACE CONDITIONS AT THE DESIGNATED BORING LOCATIONS ON THE PARTICULAR DATE DRILLED. GROUNDWATER ELEVATIONS SHOWN ON THE BORING LOGS

REPRESENT GROUNDWATER SURFACES ENCOUNTERED ON THE DATES SHOWN. FLUCTUATIONS IN WATER TABLE LEVELS SHOULD BE ANTICIPATED THROUGHOUT THE YEAR

- Water Table existing at time of boring

- SPT Boring Location

N - Standard Penetration Resistance in Blows per Foot -200 - Percent Passing the # 200 Standard Sieve MC - Percent Moisture Content

LL - Liquid Limit

PI - Plasticity Index

SP - Unified Soil Classification System (ASTM D2487) 50/2 - 50 Blows for 2-inches penetration into soil

Temporary Steel Casing

ENVIRONMENTAL CLASSIFICATION

Superstructure - Slightly Aggressive Substructure - Moderately Aggressive (Steel) Moderately Aggressive (Concrete)

CORROSION SERIES TEST RESULTS

Resistivity	Chlorides	Sulfate		
Ohms - cm	ppm	ppm	pH	
11,050	15	BDL	8.5	(Soil)
1,848	55	38	7.7	(Watér)

	GRANULAR MATERIALS						
Relative Density Very Loose Loose Medium Dense Dense Very Dense	Safety Hammer SPT N-Value Blows/Foot Less than 4 4 - 10 10 - 30 30 - 50 Greater than 50	Automatic Hammer SPT N-Value Blows/Foot Less than 3 3 - 8 8 - 24 24 - 40 Greater than 40					

	SILTS AND CLAY	′S
Consistency Very Soft Soft Firm Stiff Very Stiff Hard	Safety Hammer SPT N-Value Blows/Foot Less than 2 2 - 4 4 - 8 8 - 15 15 - 30 Greater than 30	Automatic Hammer SPT N-Value Blows/Foot Less than 1 1 - 3 3 - 6 6 - 12 12 - 24 Greater than 24

	REVISIONS					
DATE DESCRIPTION DATE DESCRIPTION						

ENGINEER OF RECORD: IVAN SOKOLIC, P.E. No. 64114 ARDAMAN AND ASSOCIATES, INC. 9970 BAVARIA ROAD FORT MYERS, FLORIDA 33913

LEE COUNTY DEPARTMENT OF TRANSPORTATION PROJECT FINANCIAL PROJECT ID ABLE CANAL PATHWAY 435351-2-58-01

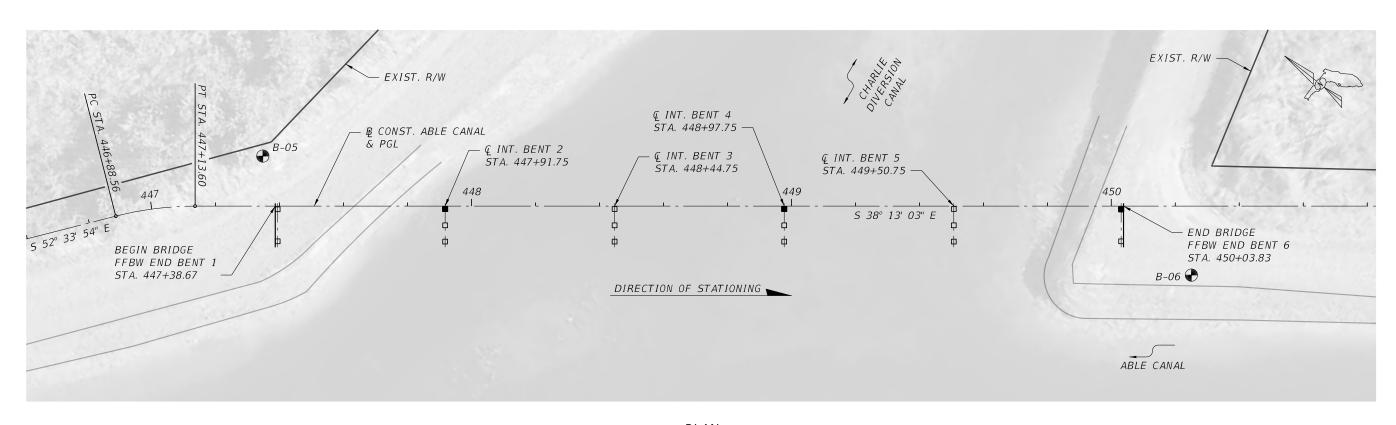
REPORT OF CORE BORINGS

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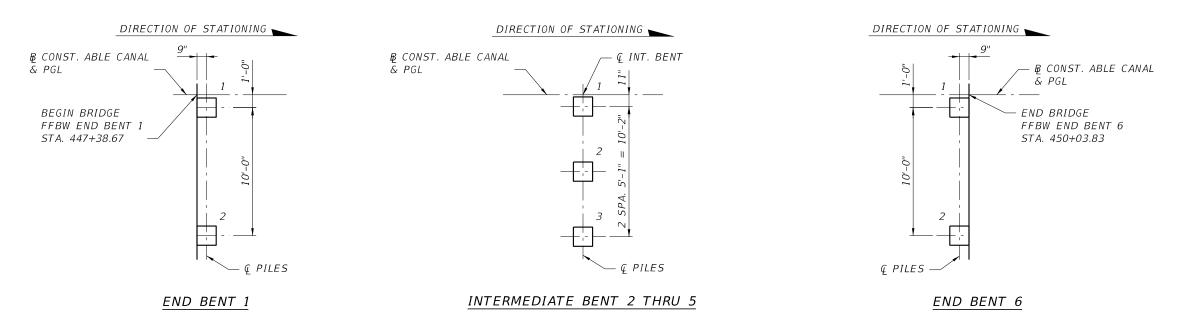
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<u>PLAN</u>

LEGEND

- □ PROPOSED 18" SQUARE PRESTRESSED CONCRETE PILE
- PROPOSED 18" SQUARE PRESTRESSED CONCRETE TEST PILE
- APPROXIMATE BORING LOCATION



ABIE CANAL	PATHWAY	OVFR	CHARLIF	DIVERSION	CANAI

	REVI:	SIONS			LEE CO	UNTY
DATE	DESCRIPTION	DATE	DESCRIPTION			
				13461 Parker Commons Blvd, Suite 104	2234 134(11:43)1 (4	111111101 01111111011
				Fort Myers, Florida 33912 KINNET CAMPO Engineer of Record: Lucio Martinez	PROJECT	FINANCIAL PROJECT ID
				KISINGER CAMPO Engineer of Record: Lucio Martinez & ASSOCIATES P.E. No.: 82380	ABLE CANAL PATHWAY	435351-2-58-01

FOUNDATION LAYOUT

SHEET NO. B-8

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	PILE DATA TABLE													Table Date 01/01/16				
	INSTALLATION CRITERIA							DESIGN CRITERIA							PILE CUT-OFF ELEVATIONS			
PIER or BENT NUMBER	PILE SIZE (in.)	NOMINAL BEARING RESISTANCE (tons)	NOMINAL UPLIFT RESISTANCE (tons)	MINIMUM TIP ELEVATION (ft.)	TEST PILE LENGTH (ft.)	REQUIRED JET ELEVATION (ft.)	REQUIRED PREFORM ELEVATION (ft.)	FACTORED DESIGN LOAD (tons)	FACTORED DESIGN UPLIFT LOAD (tons)	DOWN DRAG (tons)	TOTAL SCOUR RESISTANCE (tons)	NET SCOUR RESISTANCE (tons)	100-YEAR SCOUR ELEVATION (ft.)	Ø COMPRESSION	Ø UPLIFT	PILE 1	PILE 2	PILE 3
END BENT 1	18	97	N/A	-32	N/A	N/A	N/A	63	N/A	N/A	N/A	N/A	N/A	0.65	0.55	19.3	19.5	N/A
INT. BENT 2	18	108	N/A	- 32	75	N/A	N/A	70	N/A	N/A	N/A	N/A	2.3	0.65	0.55	19.4	19.5	19.6
INT. BENT 3	18	108	N/A	- 32	N/A	N/A	N/A	70	N/A	N/A	N/A	N/A	2.3	0.65	0.55	19.5	19.6	19.7
INT. BENT 4	18	108	N/A	- 32	75	N/A	N/A	70	N/A	N/A	N/A	N/A	2.3	0.65	0.55	19.4	19.5	19.6
INT. BENT 5	18	108	N/A	- 32	N/A	N/A	N/A	70	N/A	N/A	N/A	N/A	2.3	0.65	0.55	19.4	19.5	19.6
END BENT 6	18	97	N/A	-32	74	N/A	N/A	63	N/A	N/A	N/A	N/A	N/A	0.65	0.55	19.3	19.5	N/A

Factored Design Load + Net Scour Resistance + Down Drag

≤ Nominal Bearing Resistance

UPLIFT RESISTANCE - The ultimate side friction capacity that must be obtained below the 100 year scour elevation to resist pullout of the pile (Specify only when design requires uplift capacity). TOTAL SCOUR RESISTANCE - An estimate of the ultimate static side friction resistance provided by the scourable soil.

NET SCOUR RESISTANCE - An estimate of the ultimate static side friction resistance provided by the soil from the required preformed or jetting elevation to the scour elevation.

100-YEAR SCOUR ELEVATION - Estimated elevation of scour due to the 100 year storm event.

PILE INSTALLATION NOTES:

- 1. Contractor to verify location of all utilities prior to any pile installation activities.
- 2. Minimum Tip Elevation is required for lateral stability.
- 3. When a required jetting elevation is shown, the jet shall be lowered to the elevation and continue to operate at this elevation until the pile driving is completed. If jetting or preforming elevations differ from those shown on the table, the Engineer shall be responsible for determination of the required driving resistance.
- 4. No jetting will be allowed without the approval of the Engineer.
- 5. The Contractor should not anticipate being allowed to jet piles below the 100-year scour elevation or required jet elevation, whichever is deeper.
- 6. At each Bent, pile driving is to commence at the center of the Bent and proceed outward.
- 7. Take continuous dynamic load measurements during driving of all test piles in accordance with Section 455 of the FDOT Standard Specifications.
- 8. Provide a CTQP qualified inspector to record all pile driving activities.
- 9. Due to the potential of negative skin friction (down drag) loads due to anticipated settlement of approach embankment, portions of the end bent piles that are above existing ground line shall be wrapped with polyethylene sheeting per Section 459 of the FDOT Standard Specifications.

FINANCIAL PROJECT ID

435351-2-58-01

ABLE CANAL PATHWAY OVER CHARLIE DIVERSION CANAL

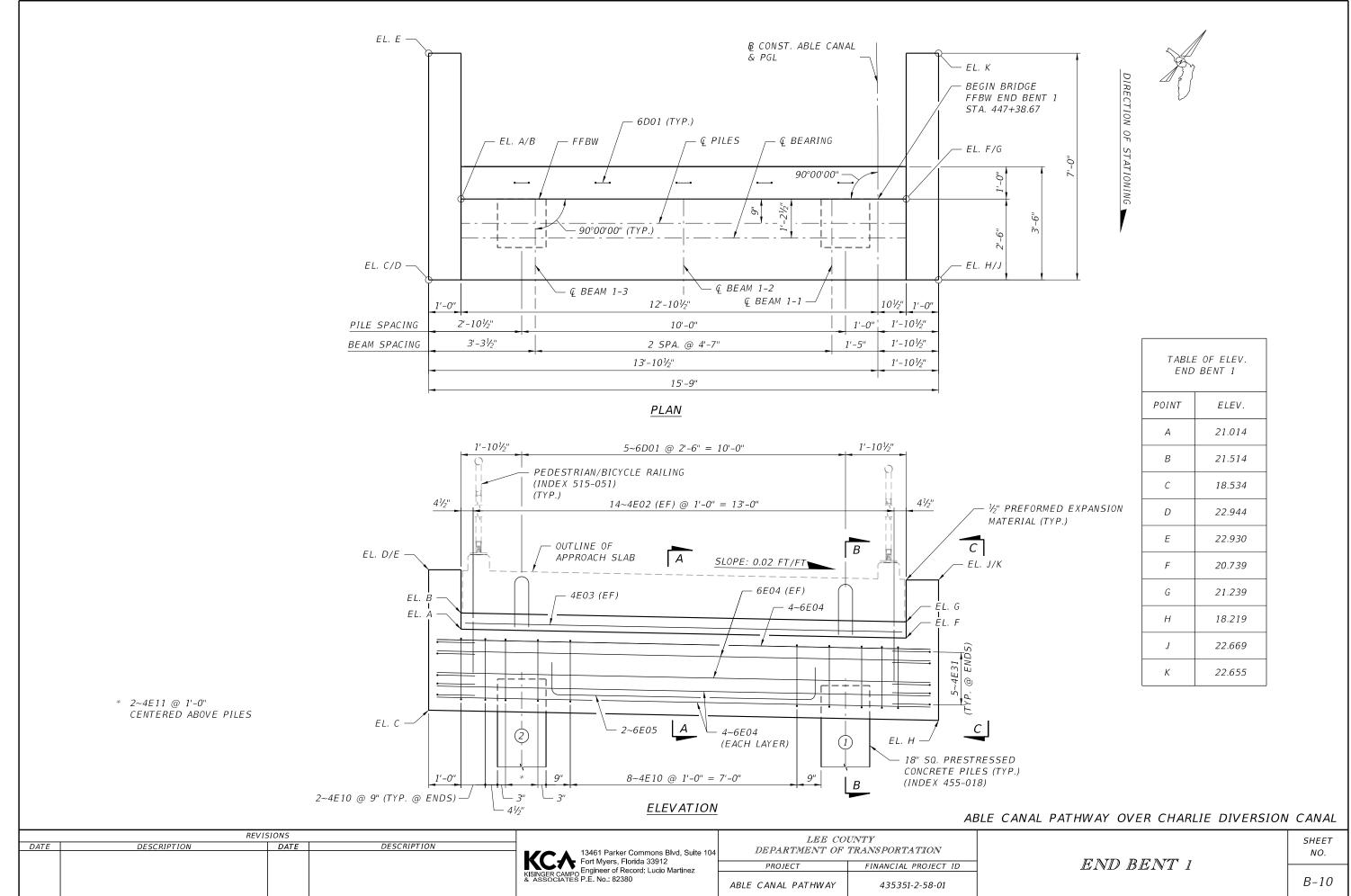
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DATE	DESCRIPTION	DATE	DESCRIPTION	
				13461 Parker Commons Blvd, Suite 1 Fort Myers, Florida 33912 KISINGER CAMPO Engineer of Record: Lucio Martinez & ASSOCIATES P.E. No.: 82380

LEE COUNTY DEPARTMENT OF TRANSPORTATION PROJECT ABLE CANAL PATHWAY

PILE DATA TABLE

SHEET NO.

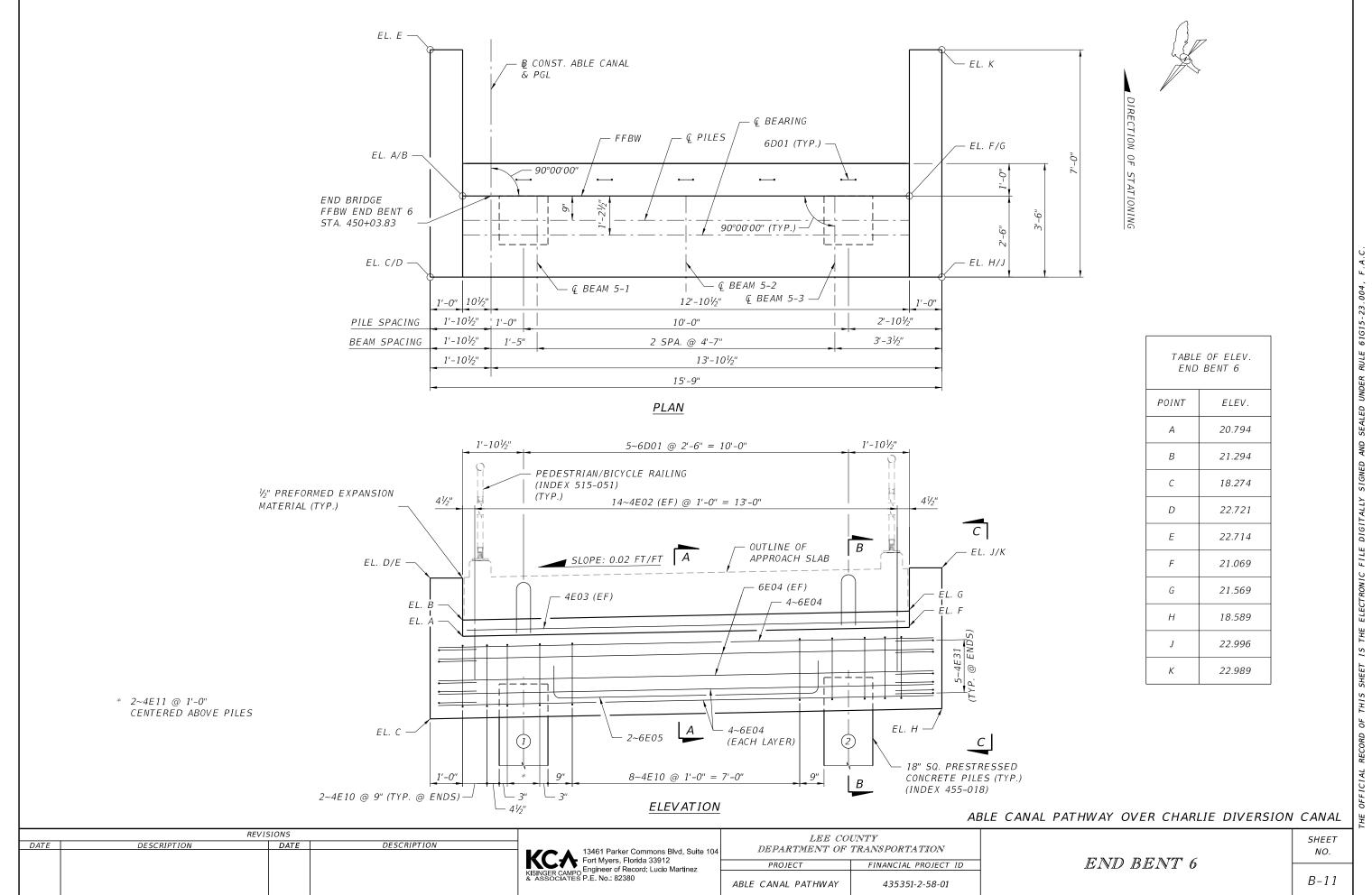
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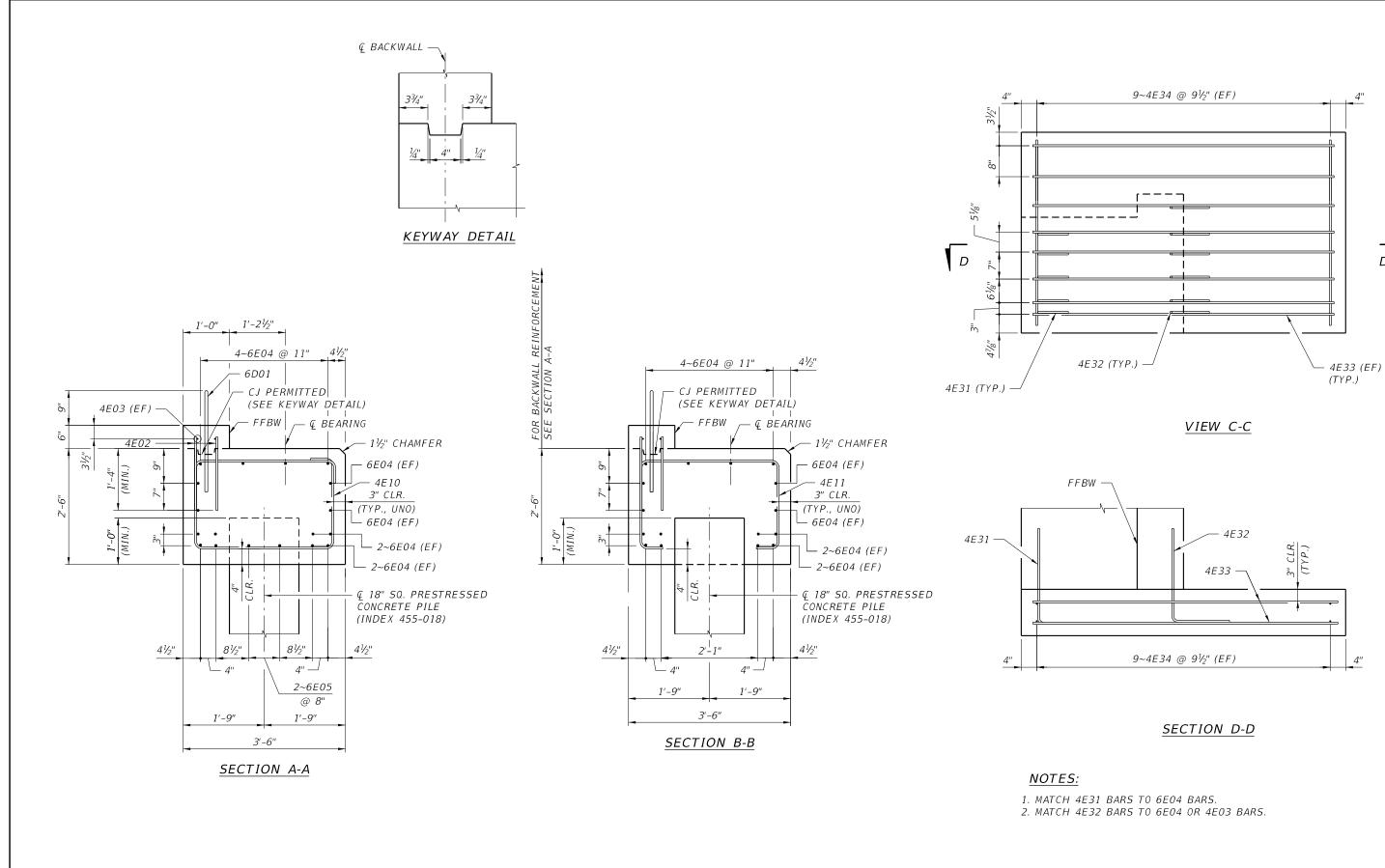
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ABLE CANAL PATHWAY OVER CHARLIE DIVERSION CANAL

REVISIONS

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DATE DESCRIPTION

LEE COUNTY
DEPARTMENT OF TRANSPORTATION

KISINGER CAMPO Engineer of Record: Lucio Martinez
ASSOCIATES P.E. No.: 82380

LEE COUNTY
DEPARTMENT OF TRANSPORTATION

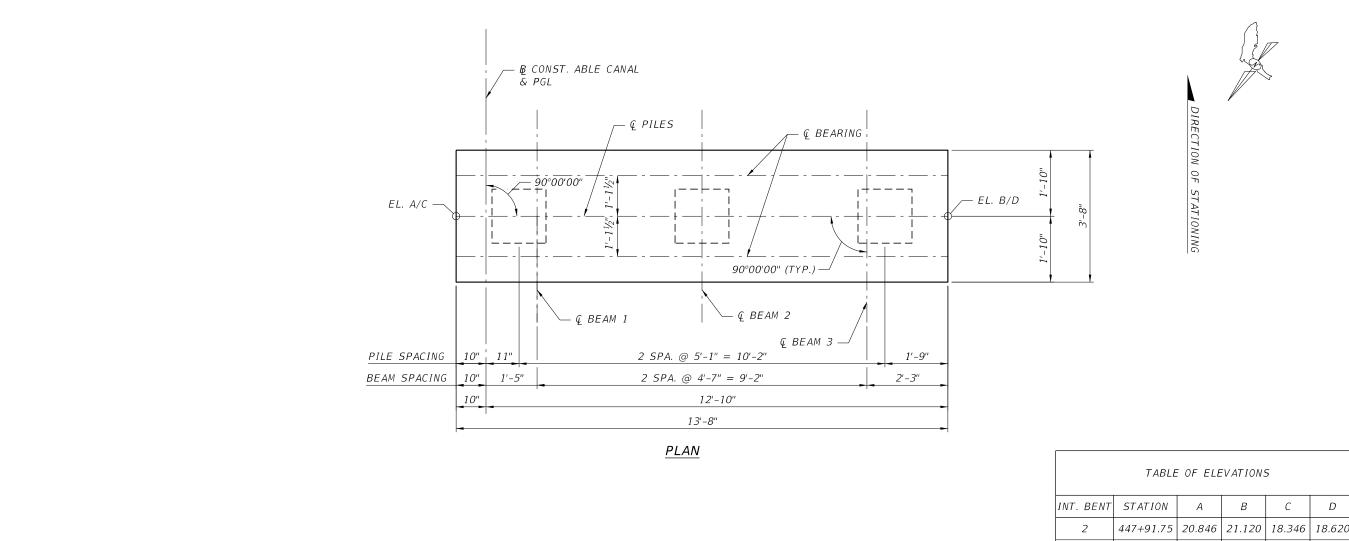
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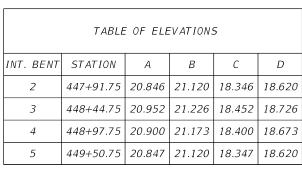
ABLE CANAL PATHWAY 435351-2-58-01

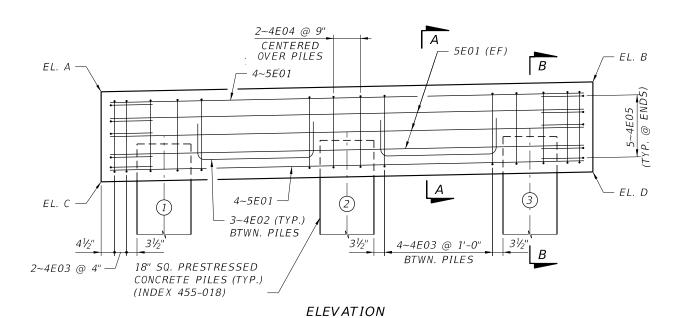
END BENT DETAILS

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ARIE CANAI	ΡΔΤΗΜΔΥ	OVFR	$CH\Delta RIIF$	DIVERSION CANAL	

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					PROJECT	FINANC
				KISINGER CAMPO Engineer of Record: Lucio Martinez & ASSOCIATES P.E. No.: 82380	ABLE CANAL PATHWAY	435

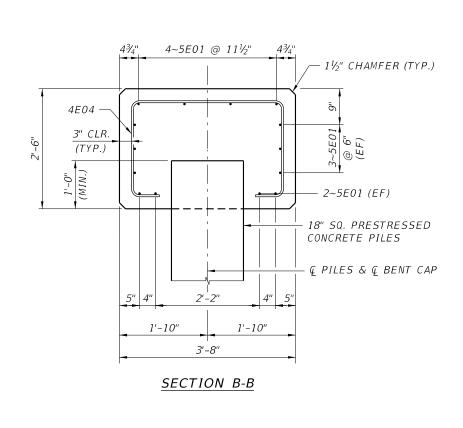
LEE COUNTY DEPARTMENT OF TRANSPORTATION						
PROJECT	FINANCIAL PROJECT ID					
ABLE CANAL PATHWAY	435351-2-58-01					

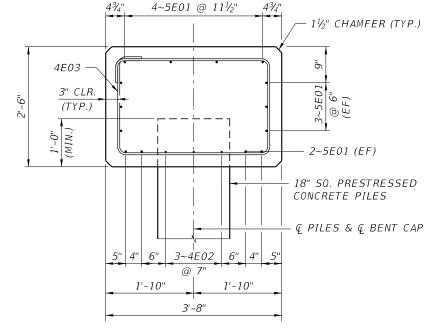
INTERMEDIATE BENTS

SHEET NO. B-13

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SECTION A-A

ABLE CANAL PATHWAY OVER CHARLIE DIVERSION CANAL

	REVISIONS DATE DESCRIPTION DESCRIPTION					
DATE	DESCRIPTION	DATE	DESCRIPTION			
				13461 Parker Commons Blvd, Suite Fort Myers, Florida 33912 KISINGER CAMPO Engineer of Record: Lucio Martinez & ASSOCIATES P.E. No.: 82380		

r Commons Blvd, Suite 104
Florida 33912
Record: Lucio Martinez
80

ABLE CANAL PATHWAY

LEE COUNTY
DEPARTMENT OF TRANSPORTATION

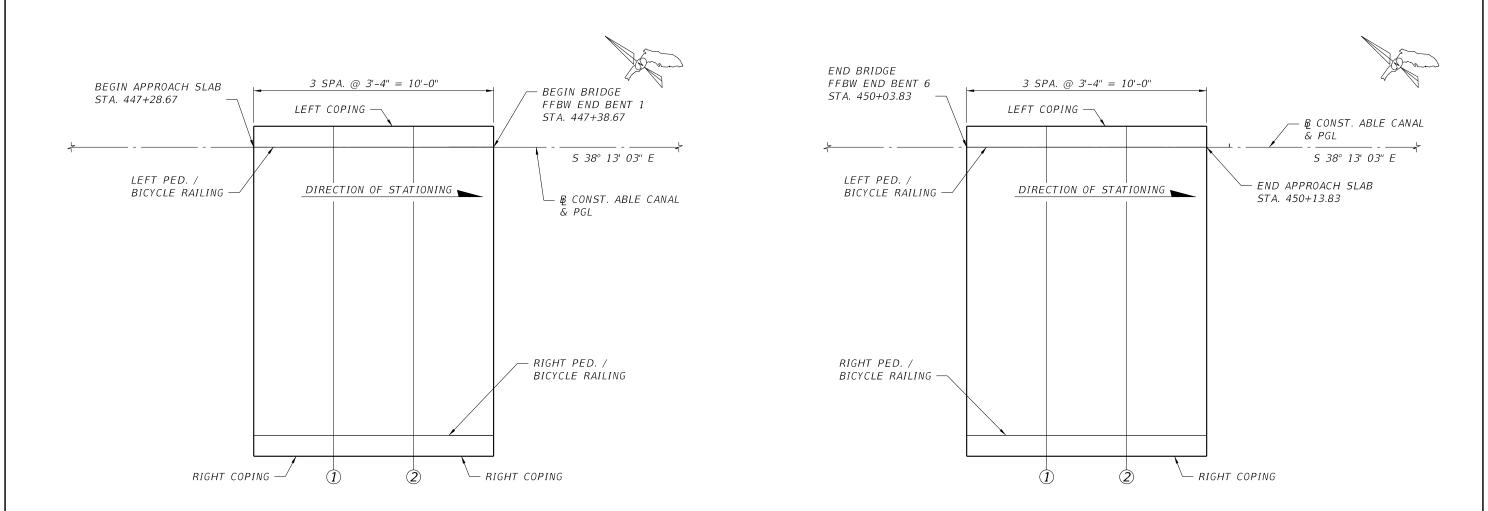
PROJECT FINANCIAL PROJECT ID

435351-2-58-01

INTERMEDIATE BENT DETAILS

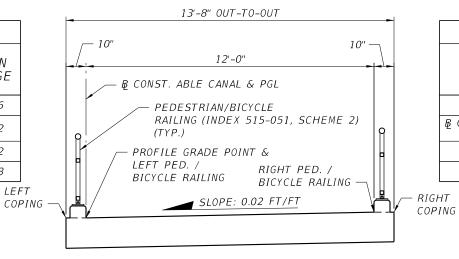
SHEET NO.

B-14



APPROACH SLAB 1

FINISH GRADE ELEVATIONS									
	BEGIN	APPR0AC	BEGIN						
LOCATION/POINTS	A.S. 1	1	2	BRIDGE					
LEFT COPING	22.636	22.650	22.659	22.666					
₽ CONST. ABLE CANAL & PGL / LEFT PED. / BICYCLE RAILING	22.652	22.666	22.675	22.682					
RIGHT PED. / BICYCLE RAILING	22.892	22.906	22.915	22.922					
RIGHT COPING	22.907	22.922	22.931	22.938					



APPROACH SLAB 2

FINISH GRADE ELEVATIONS									
	END	APPROAC.	END A.S.						
LOCATION/POINTS	BRIDGE	1	2	2					
LEFT COPING	22.720	22.717	22.713	22.706					
₽ CONST. ABLE CANAL & PGL / LEFT PED. / BICYCLE RAILING	22.736	22.733	22.729	22.722					
RIGHT PED. / BICYCLE RAILING	22.976	22.973	22.969	22.962					
RIGHT COPING	22.992	22.988	22.985	22.978					

TYPICAL SECTION THRU APPROACH SLAB

ABLE CANAL PATHWAY OVER CHARLIE DIVERSION CANAL

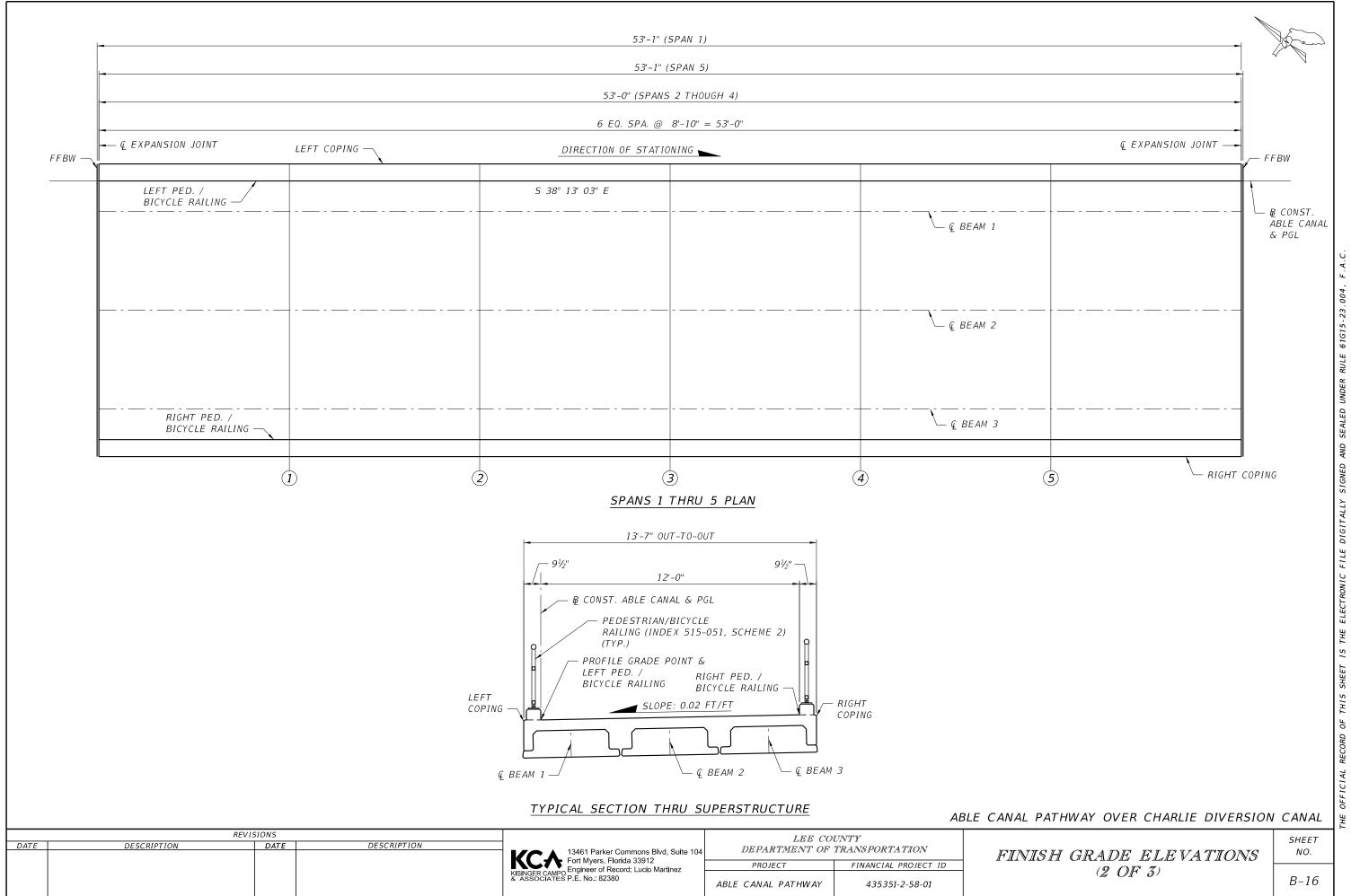
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DATE	DESCRIPTION	DATE	DESCRIPTION]	TO THE A TO STEEL STEEL OF THE		
				13461 Parker Commons Blvd, Suite 104	DB1111(111111111 O1 .	110211401 01(12111014	FINISH GRADE ELEVATIONS
				13461 Parker Commons Blvd, Suite 104 Fort Myers, Florida 33912 Fnoineer of Record Lucio Martinez	PROJECT	FINANCIAL PROJECT ID	
				KISINGER CAMPO LIIGINEER OF RECORD. EUCLO MARTINEZ & ASSOCIATES P.E. No.: 82380	ARLE CANAL RATINARY	425251 2 50 01	(1 OF 3)
					ABLE CANAL PATHWAY	435351-2-58-01	

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NO.

B-15



F.	INISH G	RADE E	LEVAT	IONS				
LOCATION (POINTS	BEGIN	SEGIN SPAN 1						
LOCATION/POINTS	BRIDGE	1	2	3	4	5	BENT 2	
LEFT COPING	22.666	22.684	22.701	22.719	22.737	22.754	22.772	
段 CONST. ABLE CANAL & PGL / LEFT PED. / BICYCLE RAILING	22.682	22.700	22.717	22.735	22.753	22.770	22.788	
Q BEAM 1	22.710	22.728	22.746	22.763	22.781	22.799	22.816	
€ BEAM 2	22.802	22.820	22.837	22.855	22.873	22.890	22.908	
€ BEAM 3	22.893	22.911	22.929	22.947	22.964	22.982	23.000	
RIGHT PED. / BICYCLE RAILING	22.922	22.940	22.957	22.975	22.993	23.010	23.028	
RIGHT COPING	22.938	22.955	22.973	22.991	23.008	23.026	23.044	

Fi	INISH G	RADE E	LEVAT	IONS			
LOCATION (BOINTS	Q INT.		€ INT.				
LOCATION/POINTS	BENT - 2	1	2	3	4	5	BENT 3
LEFT COPING	22.772	22.790	22.807	22.825	22.843	22.860	22.878
₽ CONST. ABLE CANAL & PGL / LEFT PED. / BICYCLE RAILING	22.788	22.806	22.823	22.841	22.859	22.876	22.894
₡ BEAM 1	22.816	22.834	22.852	22.869	22.887	22.905	22.922
Ç BEAM 2	22.908	22.926	22.943	22.961	22.979	22.996	23.014
Ç BEAM 3	23.000	23.017	23.035	23.053	23.070	23.088	23.106
RIGHT PED. / BICYCLE RAILING	23.028	23.046	23.063	23.081	23.099	23.116	23.134
RIGHT COPING	23.044	23.061	23.079	23.097	23.114	23.132	23.150

FINISH GRADE ELEVATIONS											
	Q INT.	NADE E	Q INT.								
LOCATION/POINTS	BENT 3	1	2	3	4	5	BENT				
LEFT COPING	22.878	22.869	22.861	22.852	22.843	22.834	22.825				
B CONST. ABLE CANAL & PGL / LEFT PED. / BICYCLE RAILING	22.894	22.885	22.876	22.868	22.859	22.850	22.841				
€ BEAM 1	22.922	22.913	22.905	22.896	22.887	22.878	22.870				
Q BEAM 2	23.014	23.005	22.996	22.988	22.979	22.970	22.961				
Q BEAM 3	23.106	23.097	23.088	23.079	23.070	23.062	23.053				
RIGHT PED. / BICYCLE RAILING	23.134	23.125	23.116	23.108	23.099	23.090	23.081				
RIGHT COPING	23.150	23.141	23.132	23.123	23.115	23.106	23.097				

FINISH GRADE ELEVATIONS													
LOCATION (POINTS	Ç INT. BENT			SPAN 4			⊊ INT. BFNT						
LOCATION/POINTS	4	1	2	3	4	5	5 5						
LEFT COPING	22.825	22.817	22.808	22.799	22.790	22.782	22.773						
段 CONST. ABLE CANAL & PGL / LEFT PED. / BICYCLE RAILING	22.841	22.832	22.824	22.815	22.806	22.797	22.789						
Q BEAM 1	22.870	22.861	22.852	22.843	22.835	22.826	22.817						
€ BEAM 2	22.961	22.952	22.944	22.935	22.926	22.917	22.909						
€ BEAM 3	23.053	23.044	23.035	23.027	23.018	23.009	23.000						
RIGHT PED. / BICYCLE RAILING	23.081	23.072	23.064	23.055	23.046	23.037	23.029						
RIGHT COPING	23.097	23.088	23.080	23.071	23.062	23.053	23.044						

FINISH GRADE ELEVATIONS														
LOCATION (BOINTS	Q INT.			SPAN 5			END							
LOCATION/POINTS	BENT 5	1	2	3	4	5	BRIDGE							
LEFT COPING	22.773	22.764	22.755	22.746	22.738	22.729	22.720							
B CONST. ABLE CANAL & PGL / LEFT PED. / BICYCLE RAILING	22.789	22.780	22.771	22.762	22.754	22.745	22.736							
€ BEAM 1	22.817	22.808	22.799	22.791	22.782	22.773	22.764							
Q BEAM 2	22.909	22.900	22.891	22.882	22.874	22.865	22.856							
Q BEAM 3	23.000	22.992	22.983	22.974	22.965	22.956	22.948							
RIGHT PED. / BICYCLE RAILING	23.029	23.020	23.011	23.002	22.994	22.985	22.976							
RIGHT COPING	23.044	23.036	23.027	23.018	23.009	23.001	22.992							

ABLE CANAL PATHWAY OVER CHARLIE DIVERSION CANAL

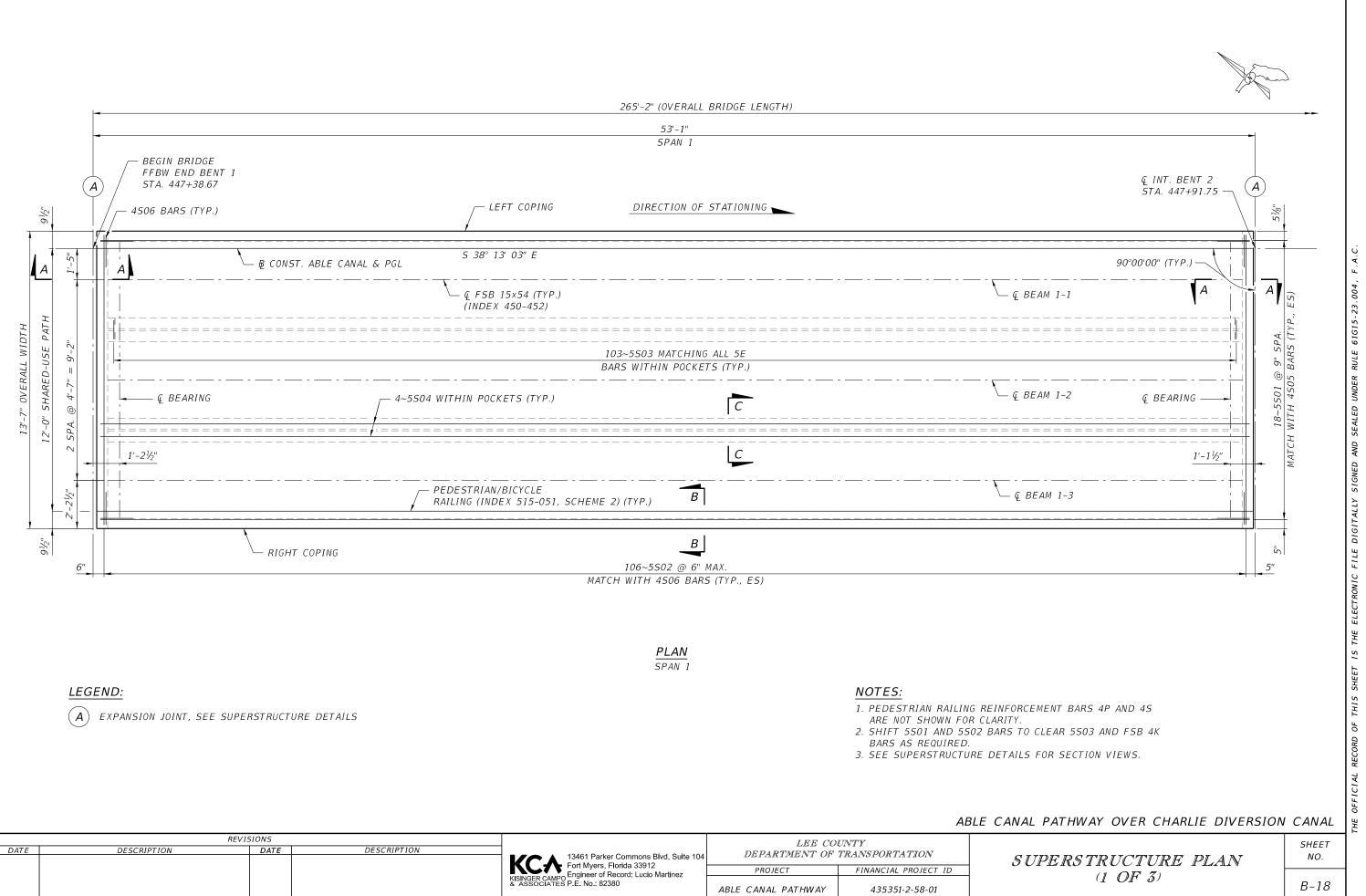
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DATE	DESCRIPTION	DATE	DESCRIPTION]
				13461 Parker Commons Blvd, Suite Fort Myers, Florida 33912 KISINGER CAMPO Engineer of Record: Lucio Martinez & ASSOCIATES P.E. No.: 82380

LEE COUNTY DEPARTMENT OF TRANSPORTATION FINANCIAL PROJECT ID PROJECT ABLE CANAL PATHWAY 435351-2-58-01

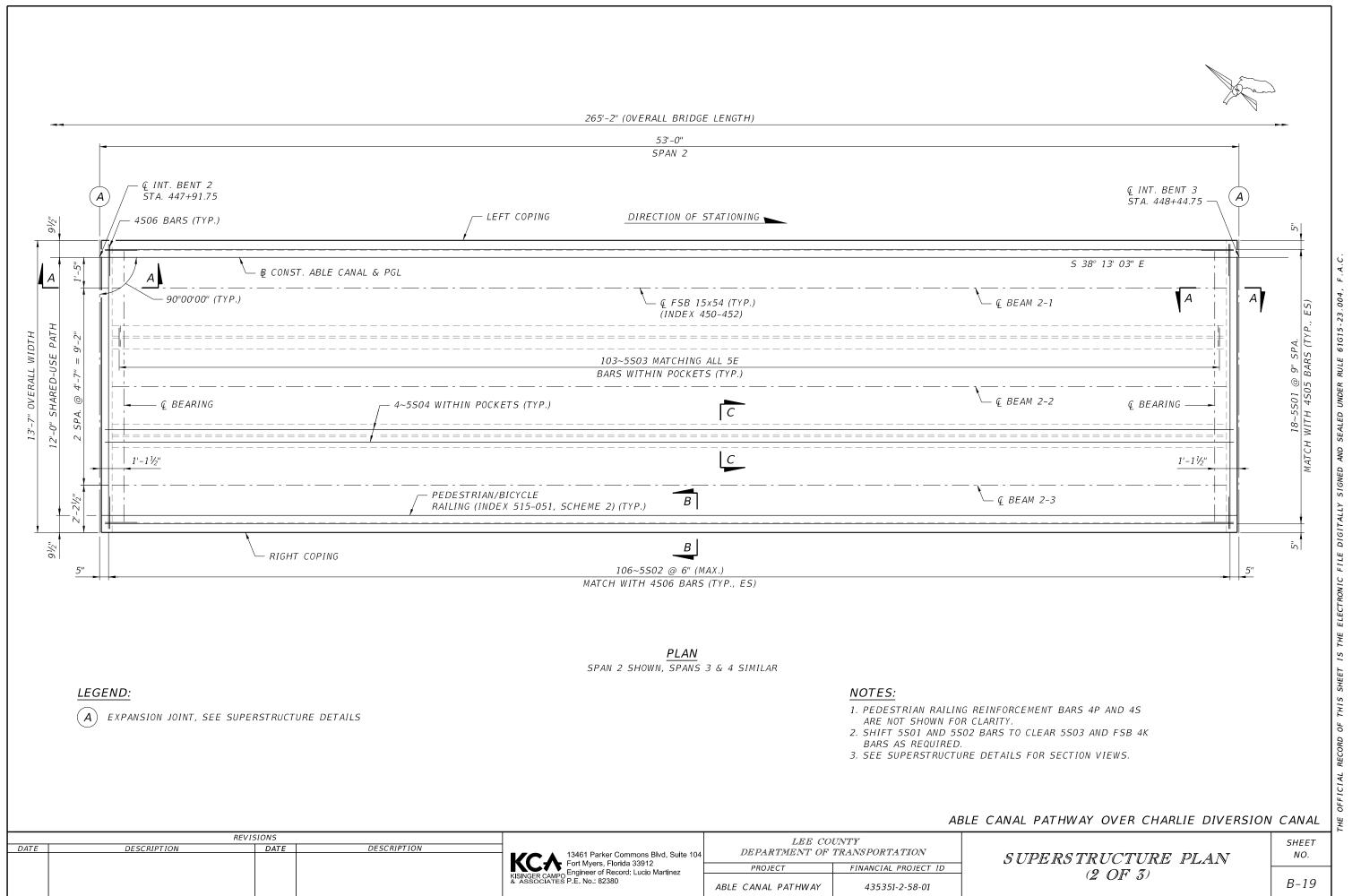
FINISH GRADE ELEVATIONS
(3 OF 3)

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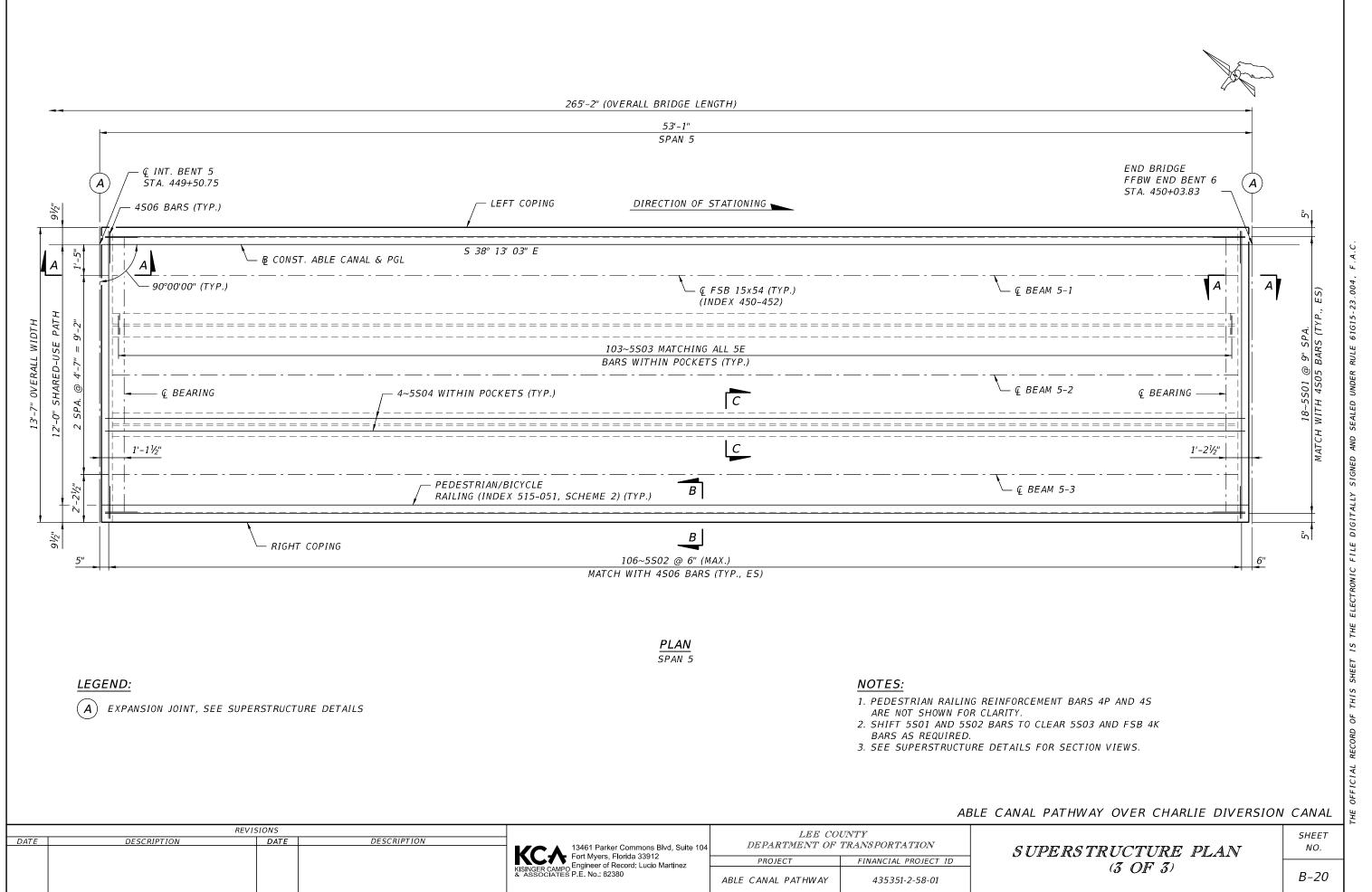
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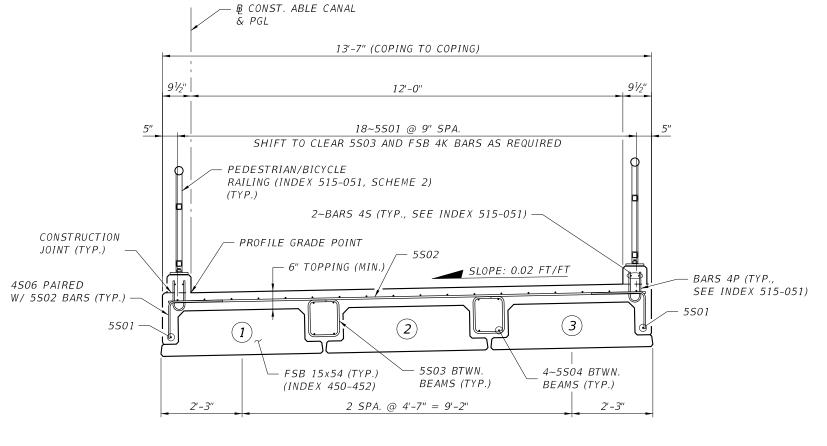
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SUPERSTRUCTURE SECTION (SPANS 1 THRU 5)

ABLE CANAL PATHWAY OVER CHARLIE DIVERSION CANAL

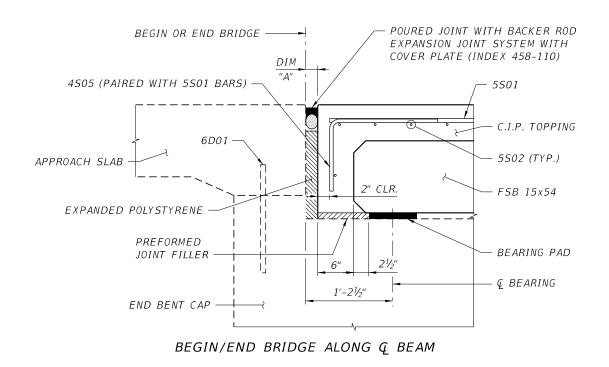
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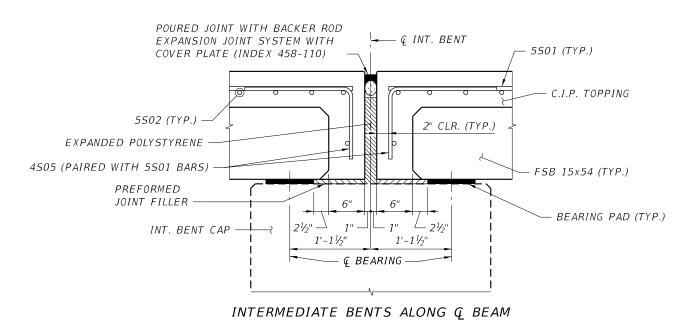
LEE COUNTY DEPARTMENT OF TRANSPORTATION PROJECT FINANCIAL PROJECT ID ABLE CANAL PATHWAY 435351-2-58-01

SUPERSTRUCTURE SECTION

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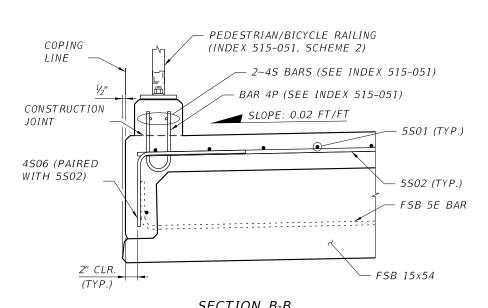
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SECTION A-A

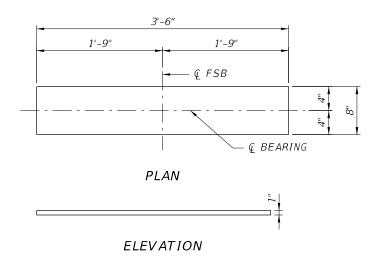
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POURED EXPANSION JOINT DATA TABLE INDEX 458-110 Table Date 1-01-09													
LOCATION	DIM. "A" @ 70°F	TOTAL DESIGN MOVEMENT		ADJUSTMENT ER 10°F									
END BENT 1	2"	1/8"		0"									
INT. BENTS 2-5	2"	³∕ ₁₆ "		0"									
END BENT 6	2"	1/8"		0"									

Dim. "A" adjustment per 10°F shown is measured perpendicular to Q Expansion Joint. Work this table with Standard Plans Index 458-110.



BEARING PAD DETAIL

NOTES:

- 1. SEE SUPERSTRUCTURE PLAN SHEETS FOR LOCATIONS OF SECTIONS A-A AND B-B.
- 2. PROVIDE PLAIN NEOPRENE BEARING PADS WITH A SHEAR MODULUS G= 110 PSI AND IN ACCORDANCE WITH SPECIFICATION 932.

ABLE CANAL PATHWAY OVER CHARLIE DIVERSION CANAL

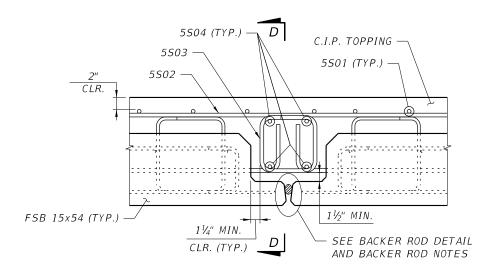
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DATE	DESCRIPTION	DATE	DESCRIPTION	1
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	PROJECT	FINANCIAL PROJECT ID
	ABLE CANAL PATHWAY	435351-2-58-01

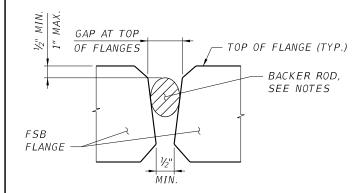
SUPERSTRUCTURE DETAILS (1 OF 2)

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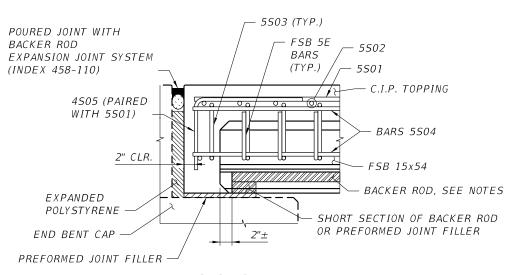
SECTION C-C



BACKER ROD DETAIL

BACKER ROD NOTES:

- 1. USE A BACKER ROD TO FORM THE BOTTOM OF THE CAST IN PLACE TOPPING AT THE GAP BETWEEN ADJACENT FSBs. USE A BACKER ROD MEETING THE REQUIREMENTS OF ASTM C1330 OR ASTM D5249, TYPES 1 OR 3, WITH A MINIMUM UNCOMPRESSED DIAMETER 50% LARGER THAN THE FIELD VERIFIED MAXIMUM WIDTH OF THE GAP BETWEEN ADJACENT FSBs. MEASURE GAP AT THE TOP OF THE FLANGES AS SHOWN IN THE BACKER ROD DETAIL.
- 2. INSTALL THE BACKER ROD FROM THE TOP DOWN TO THE POSITION SHOWN IN THE BACKER ROD DETAIL.
- 3. SECURE THE BACKER ROD TO PREVENT DISPLACEMENT DURING TOPPING CONCRETE PLACEMENT AND TO BE MORTAR TIGHT USING A COMPATIBLE CONSTRUCTION ADHESIVE.
- 4. THE BACKER ROD MAY REMAIN IN PLACE AFTER TOPPING CONCRETE PLACEMENT.

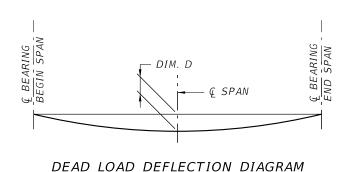


SECTION D-D

(SECTION AT BEGIN/END BRIDGE SHOWN, SECTION AT INT. BENTS SIMILAR)

CAMBER NOTE:

THE VALUES GIVEN IN THE TABLE ARE BASED ON THEORETICAL BEAM CAMBERS. THE CONTRACTOR SHALL MONITOR BEAM CAMBERS FOR THE PURPOSE OF PREDICTING CAMBER VALUES AT THE TIME OF THE TOPPING CASTING. IF THE PREDICTED CAMBERS BASED ON FIELD MEASUREMENTS DIFFER MORE THAN $\pm \frac{1}{2}$ " FROM THE THEORETICAL "NET BEAM CAMBER @ 120 DAYS" SHOWN IN THE TABLE, PROPOSE MODIFIED DIMENSIONS AS REQUIRED AND SUBMIT TO THE ENGINEER FOR APPROVAL A MINIMUM OF 21 DAYS PRIOR TO CASTING TOPPING CONCRETE.



CAMI	Table Date 11-01-20			
LOCA	TION	NET BEAM CAMBER (PRESTRESS - DEAD LOAD		DIM. D AD DEFLECTION
SPAN NO.	BEAM NO.	•	DUE TO T	OPPING CASTING O DAYS (in.)
1-5	1 AND 3	2³/ ₁₆ "		11/16"
1-5	2	2³/ ₁₆ "	1 ½"	

NOTE:

435351-2-58-01

SEE SUPERSTRUCTURE PLAN SHEETS FOR LOCATION OF SECTION C-C.

ABLE CANAL PATHWAY OVER CHARLIE DIVERSION CANAL

	REVI.			
DATE	DESCRIPTION	DATE	DESCRIPTION]
				13461 Parker Commons Blvd, Suite 104 Fort Myers, Florida 33912 KISINGER CAMPO Engineer of Record: Lucio Martinez & ASSOCIATES P.E. No.: 82380

LEE COUNTY
DEPARTMENT OF TRANSPORTATION
PROJECT FINANCIAL PROJECT ID

ABLE CANAL PATHWAY

SUPERSTRUCTURE DETAILS
(2 OF 2)

SHEET NO.

B-23

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FLORIDA SLAB	BEAM -	<i>TABLE</i>	OF	<i>VARIABLES</i>
--------------	--------	--------------	----	------------------

FLORIDA SLAB BEAM - TABLE OF VARIABLES											able Date	11-01-20																		
LOCA	ATION	55.444		RETE PRO					44/0	1 F 0		BEAM									RI	EINFO	RCING	STEEL				<u> </u>		
SPAN	BEAM	BEAM TYPE	CLASS	STRENGT 28 Day	ΓΗS (psi,) PTRN.	. <u> </u>	ASE	ANG	LE Ø	D.	MENSION			3 <i>C</i>	4D1	4D2		4D3		5E1		5E2	6Y1	6Y2	4K	NO. OF BA	AR SPACES	BAR SF	PACING*
NO.	NO.		CLASS	28 Day		TYPE	END 1	END 2	END 1	END 2				NO.				NO.	DIM D	NO.	DIM E	_				NO.	51	52	V1	V2
1 - 5	1 & 3	FSB 15x54	VI	8500	6000	1	1	1	90	90	4'-6"	52'-5 ⁷ / ₈ "	<i>5</i> ⁄8"	53	4'-11/2"	2'-3"	2'-3"	48	2'-3"	-	-	103	4'-21/2"	3'-11/2"	3'-11/2"	132	5	10	12"	24"
1 - 5	2	FSB 15x54	VI	8500	6000	1	1	1	90	90	4'-6"	52'-5 ⁷ / ₈ "	5/8"	<i>53</i>	4'-11/2"	2'-3"	2'-3"	48	2'-3"	103	4'-4"	_	_	3'-11/2"	3'-11/2"	132	5	10	12"	24"

NOTE: WORK THIS SHEET WITH STANDARD PLANS INDEX 450-450 THROUGH 450-453.

STRAND DEBONDING LEGEND

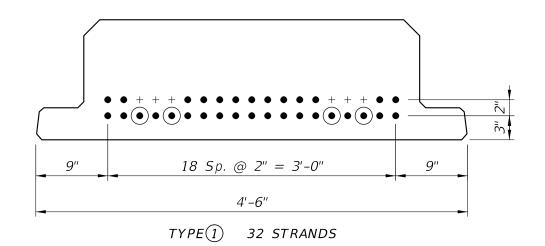
• - FULLY BONDED STRANDS.

• - STRANDS DEBONDED 5'-0" FROM END OF BEAM.

NOTE: ON BEAMS WITH SKEWED ENDS THE DEBONDED LENGTH SHALL BE MEASURED ALONG THE DEBONDED STRAND.

DIMENSION NOTES

* ALL LONGITUDINAL BEAM DIMENSIONS SHOWN ON THIS SHEET WITH A SINGLE ASTERISK (*) ARE MEASURED ALONG THE TOP OF BEAM AT THE CENTERLINE. DIMENSION "R" IS CALCULATED AT MID-HEIGHT OF THE BEAM.



STRAND DESCRIPTION: USE 0.6" DIAMETER, GRADE 270, LOW RELAXATION CARBON STEEL STRANDS STRESSED AT 43.94 KIPS EACH. AREA PER STRAND EQUALS 0.217 SQ. IN.

===== STRAND PATTERNS=====

ABLE CANAL PATHWAY OVER CHARLIE DIVERSION CANAL

	REVI	SIONS		
DATE	DESCRIPTION	DATE	DESCRIPTION]
				13461 Parker Commons Blvd, Suite Fort Myers, Florida 33912 KISINGER CAMPO Engineer of Record: Lucio Martinez & ASSOCIATES P.E. No.: 82380

13461 Parker Commons Blvd, Suite 104	LEE CO DEPARTMENT OF	UNTY TRANSPORTATION
Fort Myers, Florida 33912	PROJECT	FINANCIAL PROJECT ID
Engineer of Record: Lucio Martinez S P.E. No.: 82380	ABLE CANAL PATHWAY	435351-2-58-01

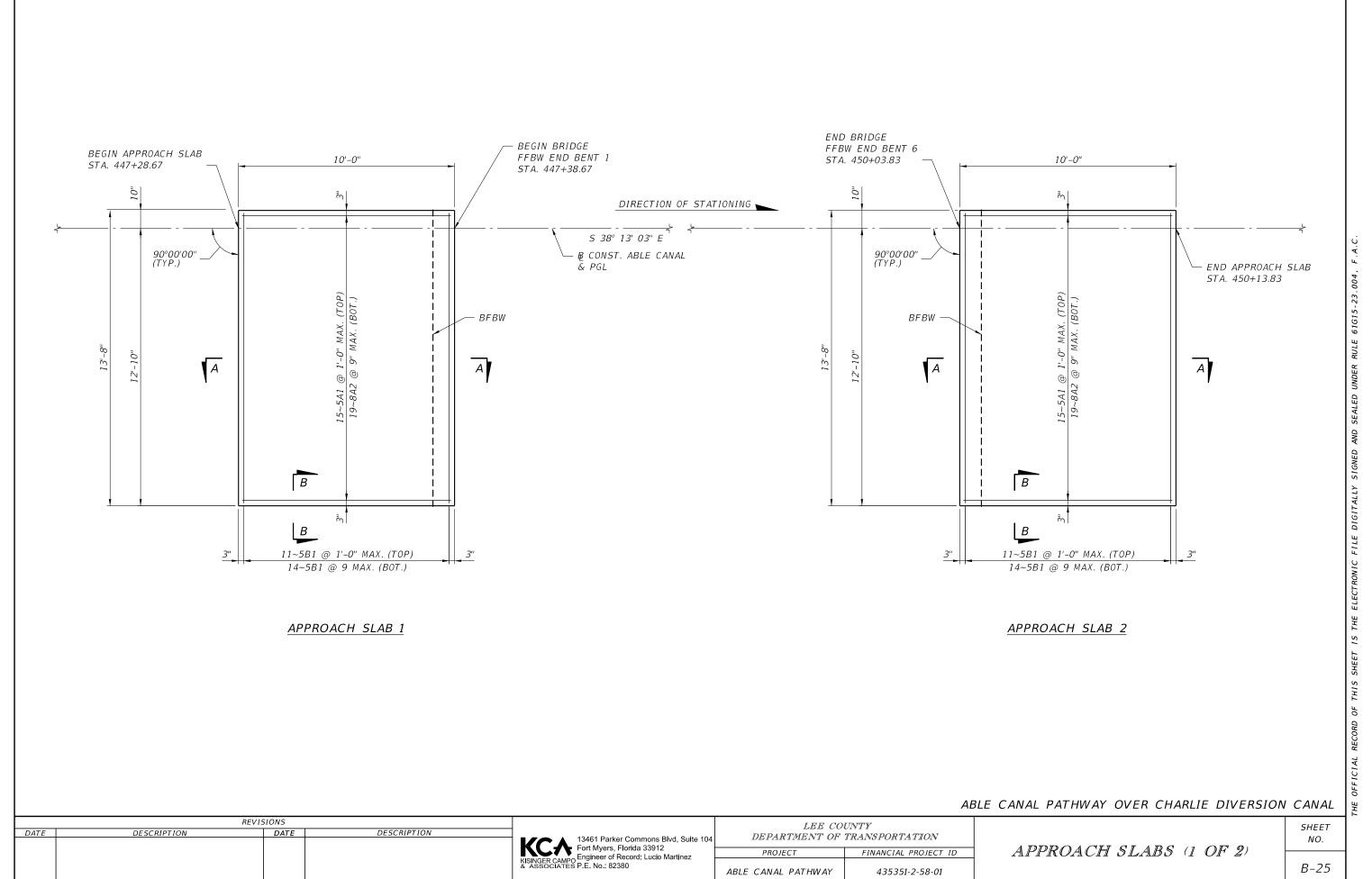
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TABLE OF BEAM VARIABLES

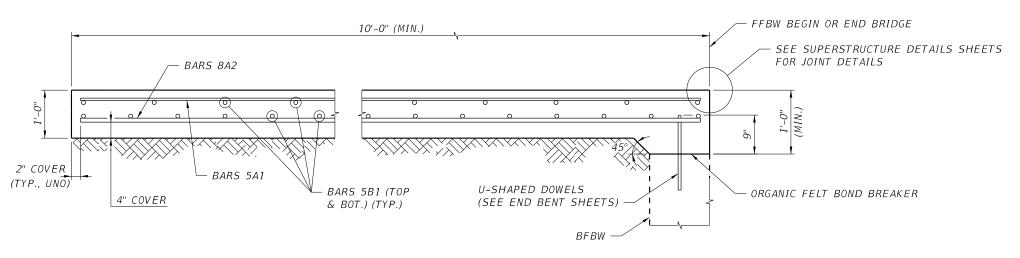
SHEET NO.

B-24

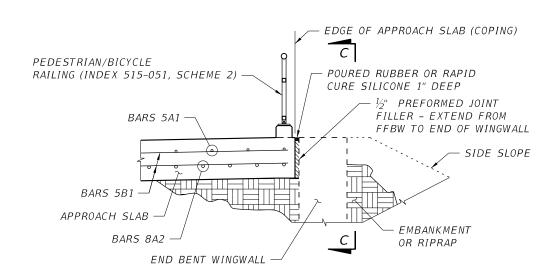
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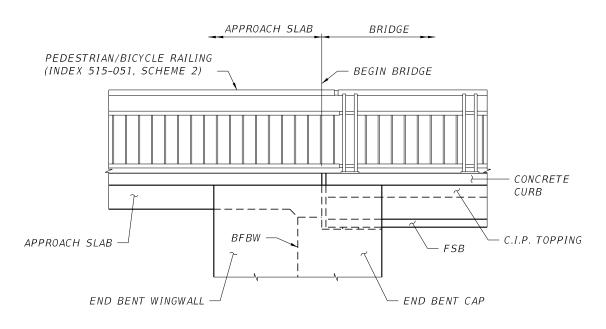
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SECTION A-A



SECTION B-B



VIEW C-C

ABLE CANAL PATHWAY OVER CHARLIE DIVERSION CANAL

	REVIS	SIONS			Ī
DATE	DESCRIPTION	DATE	DESCRIPTION	13461 Parker Commons Blvd, Suite 104 Fort Myers, Florida 33912 KISINGER CAMPO Engineer of Record: Lucio Martinez & ASSOCIATES P.E. No.: 82380	_
				& ASSOCIATES P.E. No.: 82380	

LEE COUNTY
DEPARTMENT OF TRANSPORTATION

PROJECT FINANCIAL PROJECT ID

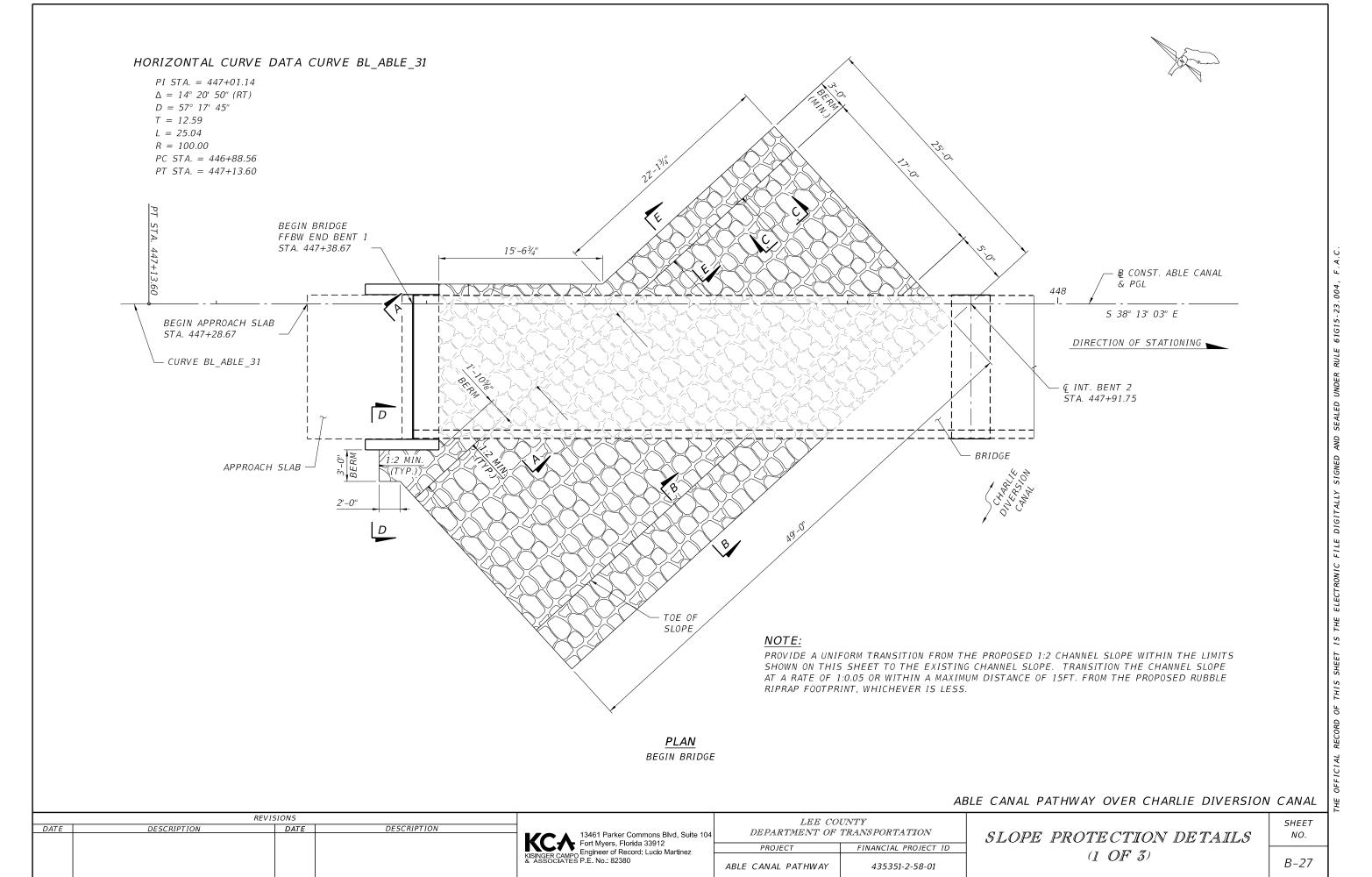
ABLE CANAL PATHWAY 435351-2-58-01

APPROACH SLABS (2 OF 2)

SHEET NO. B-26

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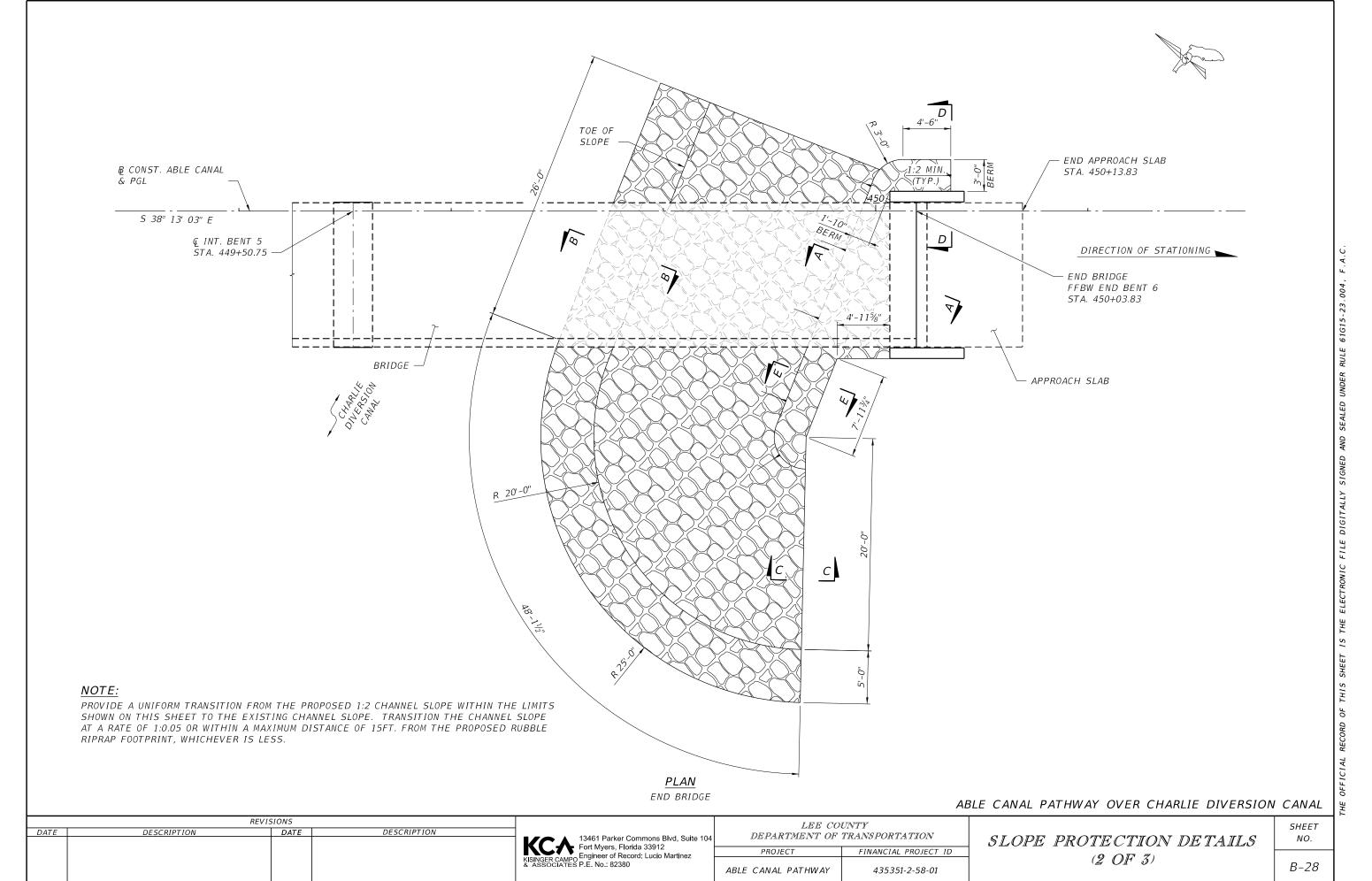
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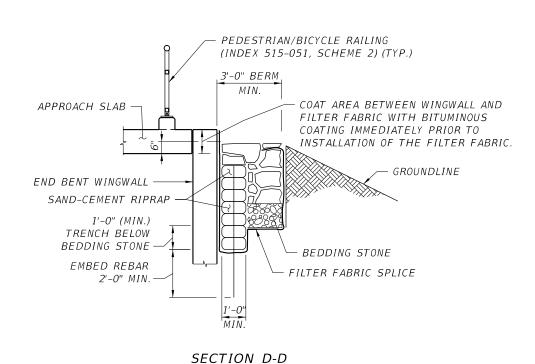
ez 8/29/2024 3:

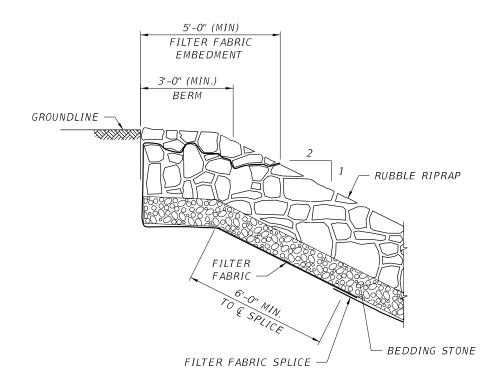
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SECTION A-A

* 3'-0" MINIMUM BERM EXCEPT AS SHOWN ON SLOPE PROTECTION DETAILS (1 OF 3) AND SLOPE PROTECTION DETAILS (2 OF 3).





SECTION E-E

ABLE CANAL PATHWAY

ABLE CANAL PATHWAY OVER CHARLIE DIVERSION CANAL

	REVI.	SIONS		
DATE	DESCRIPTION	DATE	DESCRIPTION	1
				KISINGER CAMPO EI KISINGER CAMPO EI KASSOCIATES P.

13461 Parker Commons Blvd, Suite 104 Fort Myers, Florida 33912 Engineer of Record: Lucio Martinez P.E. No.: 82380

LEE COUNTY DEPARTMENT OF TRANSPORTATION FINANCIAL PROJECT ID PROJECT

435351-2-58-01

SLOPE PROTECTION DETAILS (3 OF 3)

SHEET NO. B-29

EXISTING

BEDDING STONE

GROUNDLINE

- 2'-6" (MIN.)

8/29/2024 3:37:33 PM Default c:\pww\kca_prod\dms36828\B1RiprapRubbleDet01.dgn

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MA	RK	LENGTH	NO	TYP	STY	В	С	D	E	F	Н	J	К	N	Ø
SIZE	DES	FT IN	BARS	BAR	A G	FT IN FR	FT IN FR	FT IN FR	FT IN FR	FT IN FR	FT IN FR	FT IN FR	FT IN FR	NO	ANG
	LOCATION END BENT 1 AND 6 NO. REQUIRED = 2														
6	D01	3- 3	5	23		1 - 4	0- 2 1/4	1 - 4							
4	E02	1 - 7	28	1		1 - 7									
4	E03	15- 3	2	1		15- 3									
6	E04	15- 3	16	1		15- 3									
6	E05	10- 0	2		3 3										
4	E10	10- 7	12		4 4	1 – 1 1	3- 0								
4	E11	7-10	4	5		1-11	3- 0	0- 6	0- 6						
4	E31	2-11	10			2- 3									
4	E32	2-11	12		3	2- 3									
4	E33	6- 6	32	1		6- 6									
4	E34	3- 9	36	1		3- 9									
		1.004	TION	7 . /	<u> </u>	ENTS 2 TU	DOLLOU E			NO DEOUT					
		LOCA		1/V	<i>1. B</i>	ENTS 2 TH	RUUGH 5		/	NO. REQUII 	RED = 4				
5	E01	13- 2	14	1	\vdash	13- 2									
4	E02	4- 5	6		3 3										
4	E03	11-1	12		4 4		3- 2								
4	E04	8- 4	4	5		2- 0	3- 2	0- 7	0- 7						
4	E05	4- 4	10	18	3 3	3- 0									
		L DCA	TION	AP	PROA	CH SLABS	1 & 2		1	NO. REQUI	RED = 2				
5	A 1	9- 8	15	1		9- 8									
8	A2	9- 8	19	1		9- 8									
5	B1	13- 4	25	1		13-8									
		LOCA	TION	SP,	ANS T	1 THROUGH	5		<i></i>	NO. REQUI:	RED = 5				
5	S01	52- 6	20	1		52- 6									
5	502	13- 3	108	1		13- 3									-
5	503	4- 7	206		4 4		0- 9 1/2								-
5	504	52- 6	8	1		52- 6	72								-
4	S05	2- 9	36			1- 6	1- 3								
4	506	2- 5	212	10		1-6	0-11								
								END OF L	IST	1			ı		$\neg \neg$
E110 01 E101															

ABLE CANAL PATHWAY OVER CHARLIE DIVERSION CANAL

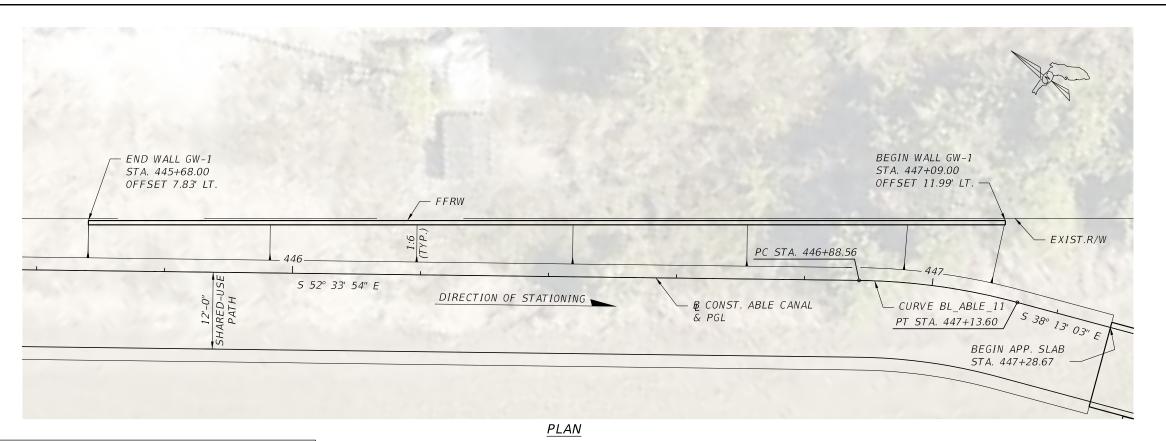
	R	EVISIONS		
DATE	DESCRIPTION	DATE	DESCRIPTION	The state of the s
				13461 Parker Commons Blvd, Suite 1 Fort Myers, Florida 33912 KISINGER CAMPO Engineer of Record: Lucio Martinez & ASSOCIATES P.E. No.: 82380

LEE COUNTY DEPARTMENT OF TRANSPORTATION PROJECT FINANCIAL PROJECT ID ABLE CANAL PATHWAY 435351-2-58-01

REINFORCING BAR LIST

SHEET NO. B-30





WALL ELEVATIONS									
			EXISTING	TOP	ВОТТОМ				
POINT	STATION	<i>OFFSET</i>	GROUNDLINE	OF WALL	OF WALL				
			ELEVATION	ELEVATION	ELEVATION				
		(FT.)	(FT.)	(FT.)	(FT.)				
1	445+68.00	7.83	18.276	18.268	17.276				
2	445+80.00	7.98	18.013	18.273	17.013				
3	446+00.00	8.24	17.787	18.467	16.787				
4	446+20.00	8.49	17.518	18.891	16.518				
5	446+40.00	8.75	17.295	19.488	16.295				
6	446+60.00	9.00	17.223	20.096	16.223				
7	446+80.00	9.26	17.056	20.697	16.056				

19.854

21.029

18.854

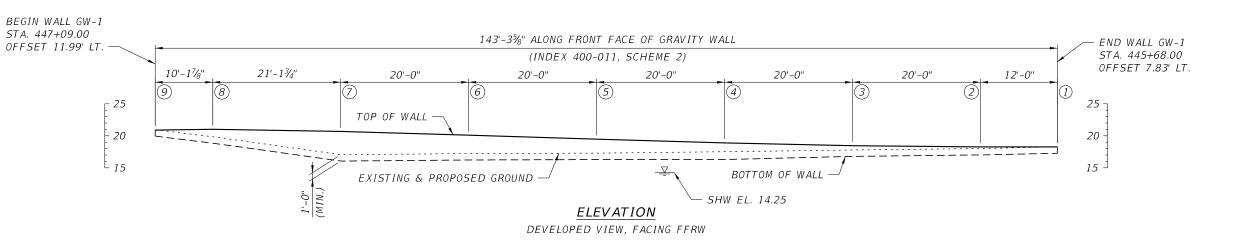
10.25

447+00.00

447+09.00

HORIZONTAL CURVE DATA CURVE BL_ABLE_11

PI STA. = 447+01.14 $\Delta = 14^{\circ} \ 20^{\circ} \ 50^{\circ} \ (RT)$ $D = 57^{\circ} \ 17^{\circ} \ 45^{\circ}$ T = 12.59 L = 25.04 R = 100.00PC STA. = 446+88.56PT STA. = 447+13.60

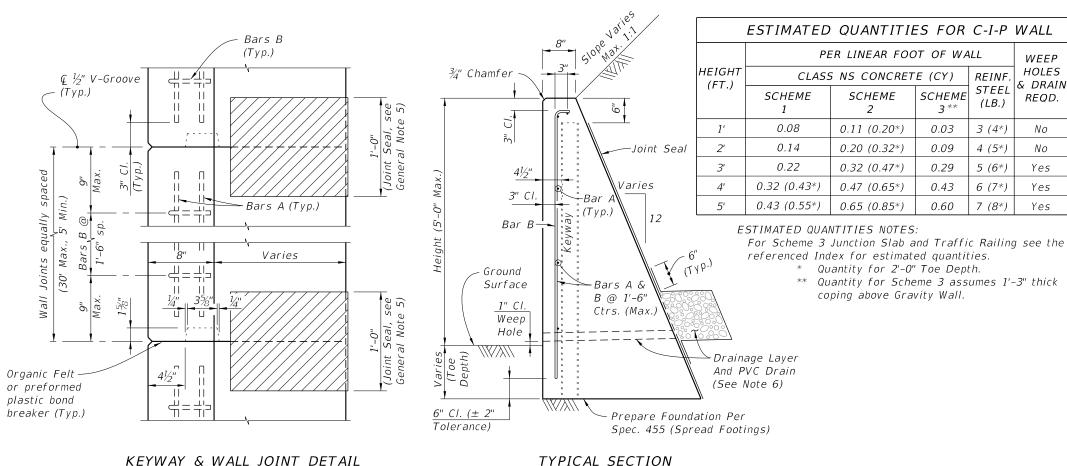


ABLE CANAL PATHWAY OVER CHARLIE DIVERSION CANAL

	REVI	SIONS			LEE CO.	UNTY
DATE	DESCRIPTION	DATE	DESCRIPTION	13461 Parker Commons Blvd, Suite 104		TRANSPORTATION
				13461 Parker Commons Blvd, Suite 104 Fort Myers, Florida 33912 KISINGER CAMPO Engineer of Record: Lucio Martinez & ASSOCIATES P.E. No.: 82380	PROJECT	FINANCIAL PROJECT ID
				KISINGER CAMPO Engineer of record Leads Manufactures P.E. No.: 82380	ABLE CANAL PATHWAY	435351-2-58-01

GRAVITY WALL

SHEET NO. B-31



(TOP VIEW)

TYPICAL SECTION C-I-P CONCRETE GRAVITY WALL

GENERAL NOTES

WEEP

HOLES

& DRAIN

REQD.

No

Yes

Yes

Yes

REINF

SCHEME

0.03

0.09

0.29

0.43

0.60

3 **

SCHEME

2

0.11 (0.20*)

0.20 (0.32*)

0.32 (0.47*)

0.47 (0.65*)

0.65 (0.85*)

STEEL

(LB.)

3 (4*)

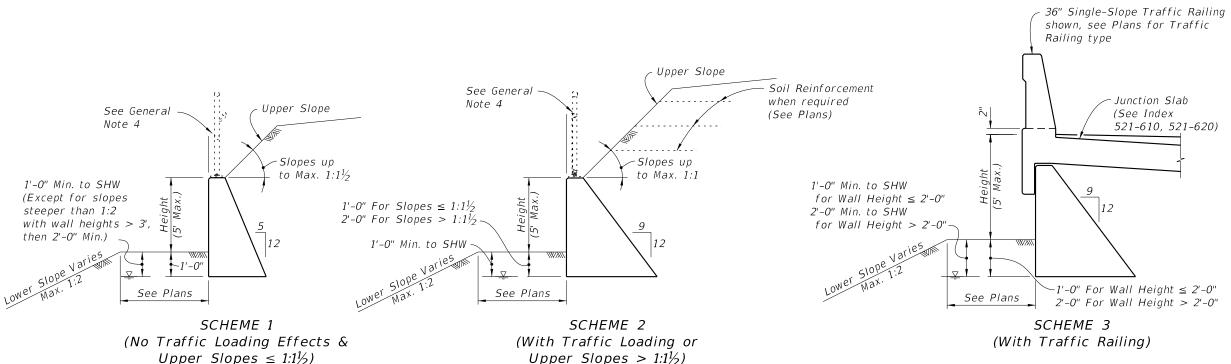
4 (5*)

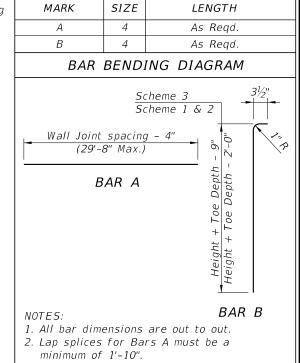
5 (6*)

6 (7*)

7 (8*)

- 1. C-I-P Gravity Walls constructed as extensions of reinforced concrete retaining walls, except for walls of proprietary designs, provide the same face texture and finish as the reinforced concrete retaining wall.
- 2. Use Class NS Concrete for Gravity Wall per Specification 347. Concrete for Scheme 3 Junction Slab and Traffic Railing shall be Class II per Specification 346, unless otherwise specified in the plans.
- 3. Reinforcing steel meets the requirements of Specification 931 (Grade 40 or 60). Smooth or Deformed Welded Wire Reinforcement (WWR) may be substituted on an equal area basis. Do not increase bar/wire spacing for Grade 60 reinforcing steel or WWR.
- 4. When required, for adjunct guiderail, see Index 515-070 or 515-080 as appropriate. For adjunct Type B fence see Index 550-002.
- 5. Joint Seal: Provide and install organic bond breaker or Type D-5 geotextile in accordance with Specifications 400 and 514 respectively. Mop all contact surfaces of concrete and Organic Felt or geotextile with cut-back asphalt. Stop Organic Felt or geotextile 6" below top of wall.
- 6. Provide a continuous 1'x1' clean gravel or crushed rock drain for wall heights 3 ft. and higher. Wrap drainage layer as shown, with Type D-3 geotextile in accordance with Specification 514. Provide 8"x8" galvanized mesh with $\frac{1}{4}"$ openings, at the inside end of the PVC Drain Pipe. Provide 2" Ø PVC Drain Pipe (Sch. 40) at 10 ft. max. spacing (when Drainage Layer is required). Locate outermost edge of Drain Pipe a minimum of 2'-0" from wall joints.
- 7. Cost of reinforcing steel, face texture, finish, joint seal, drain pipes, drainage layer, galvanized mesh and geotextile to be included in the Contract Unit Price for Concrete Class NS, Gravity Wall. Cost of concrete for Junction Slab in Scheme 3, to be included in Contract Unit Price for Concrete Traffic Railing Barrier With Junction Slab. Adjunct railings or fences to be paid for separately.





BILL OF REINFORCING STEEL

DESCRIPTION: LAST REVISION 11/01/23

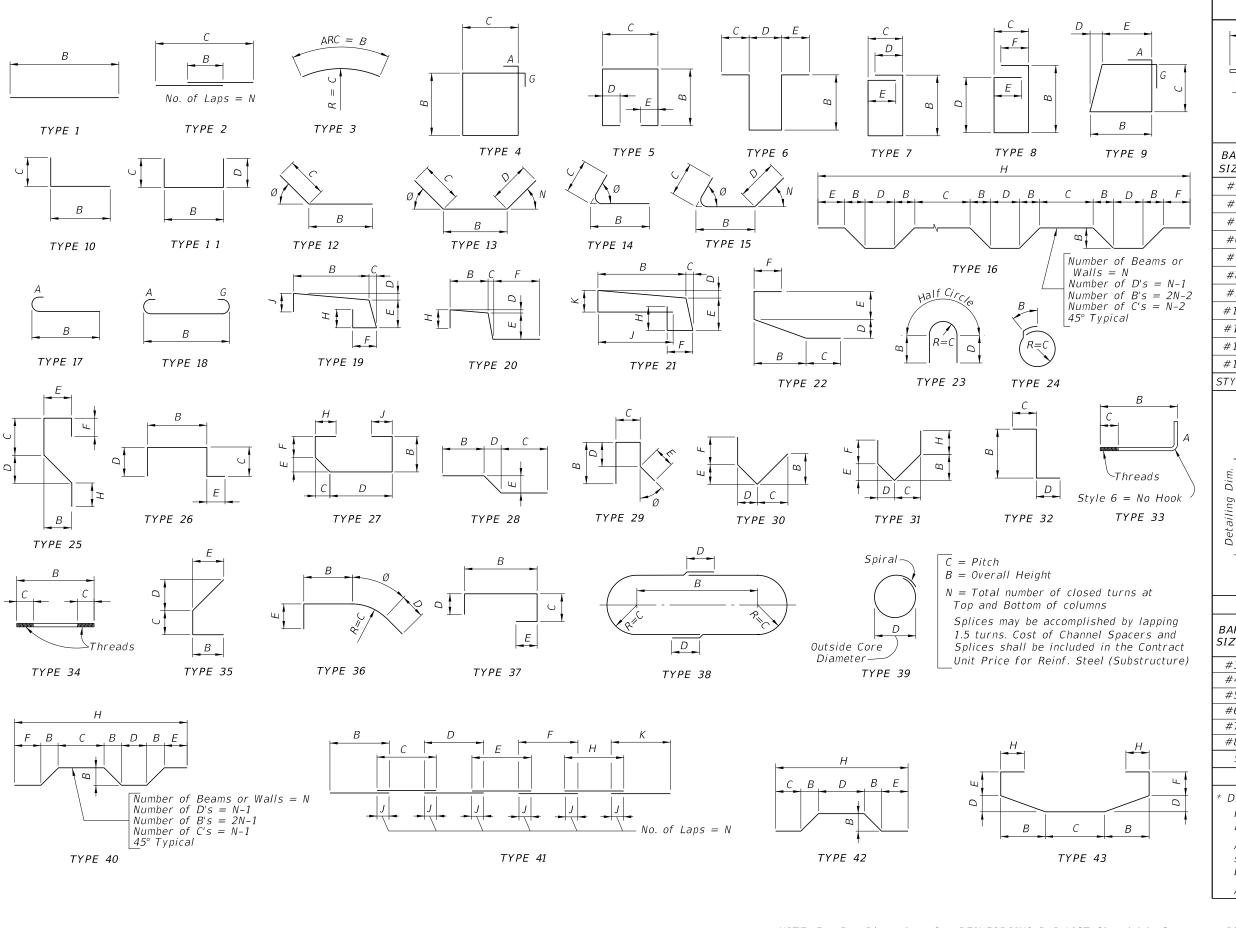
FDOT

FY 2024-25 STANDARD PLANS

GRAVITY WALL

INDEX 400-011

SHEET 1 of 1



HOOK DETAILS Detailing Dim. Detailing Dim. Hook A or G 180° 90° 180° HOOKS D SIZE A OR G J #3 21/4" 5" 3" #4 6" #5 3¾" 7" 5" #6 8" 6" #7 51/4" 10" 7" 11" 8" #8 1'-3" 11¾" #9 1'-11/4" #10 10¾" 1'-5" #11 1'-7" 1'-23/4" 12" #14 181/4" 2'-3" 1'-93/4" #18 2'-41/5" 24" 3'-0" STYLE 0 135° STIRRUPS (TIES SIMILAR) 90° HOOKS BARSIZE A or G

STIRRUP & TIE HOOK DIMENSIONS

135° HOOKS H^* A or G #3 2½" $1\frac{1}{2}$ " 4" #4 2" 41/5" 41/5" 3" 21/2" 51/2" 33/4" #6 4½" 4½" 1'-0" 5¼" 51/4" #7 1'-2" #8 6" 1'-4" 101/5" 6" STYLE 4

STYLE 6 = NO HOOK

Dimension is approximate.

Hook Styles Detailed on this sheet are for Illustration Only.

Actual Hook Style for any particular bar will be shown under A or G Heading on REINFORCING BAR LIST sheet(s) in Structures Plans.

All Dimensions are out-to-out.

NOTE: For Bar Dimensions See REINFORCING BAR LIST Sheet(s) in Structures Plans.

REVISION 11/01/20

DESCRIPTION:



90° HOOKS

6" 8"

10"

1'-0"

1'-2"

1'-4"

1'-7"

1'-10"

2'-0" 2'-7"

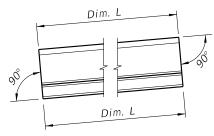
3'-5"

3

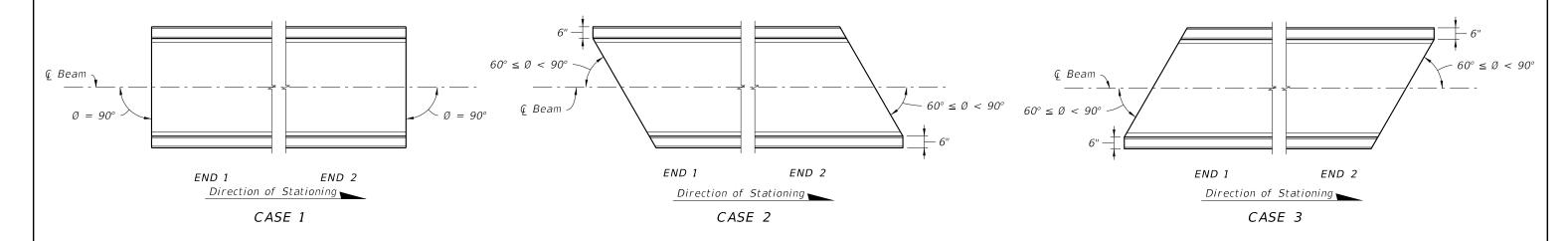
A OR G

FABRICATION NOTES

- 1. The abbreviated FSB designation for depth and width is FSB "depth" x "width", e.g. FSB 12 x 48.
- 2. All bar dimensions are out-to-out.
- 3. Strands N shall be ASTM A416, Grade 250 or 270, 3/8" Ø or larger strands, stressed to 10,000 lbs. each.
- 4. Unless otherwise noted, the minimum concrete cover for reinforcing steel shall be 2".
- 5. For referenced Dimensions, Angles and Case Numbers, see Florida Slab Beam - Table of Variables in Structures Plans.
- 6. Bars 4D1 & 6Y1 correspond to END 1, and 4D2 & 6Y2 correspond to END 2.
- 7. Bars 5E1 correspond to interior FSBs, and 5E2 correspond to exterior FSBs.
- 8. Rake the top surface of the Slab Beams transversely to provide a roughened surface with $\frac{1}{4}$ " amplitude.
- 9. Embedment of Safety Line Anchorage Devices are permitted to accommodate full protection systems. See shop drawings for details and spacings.

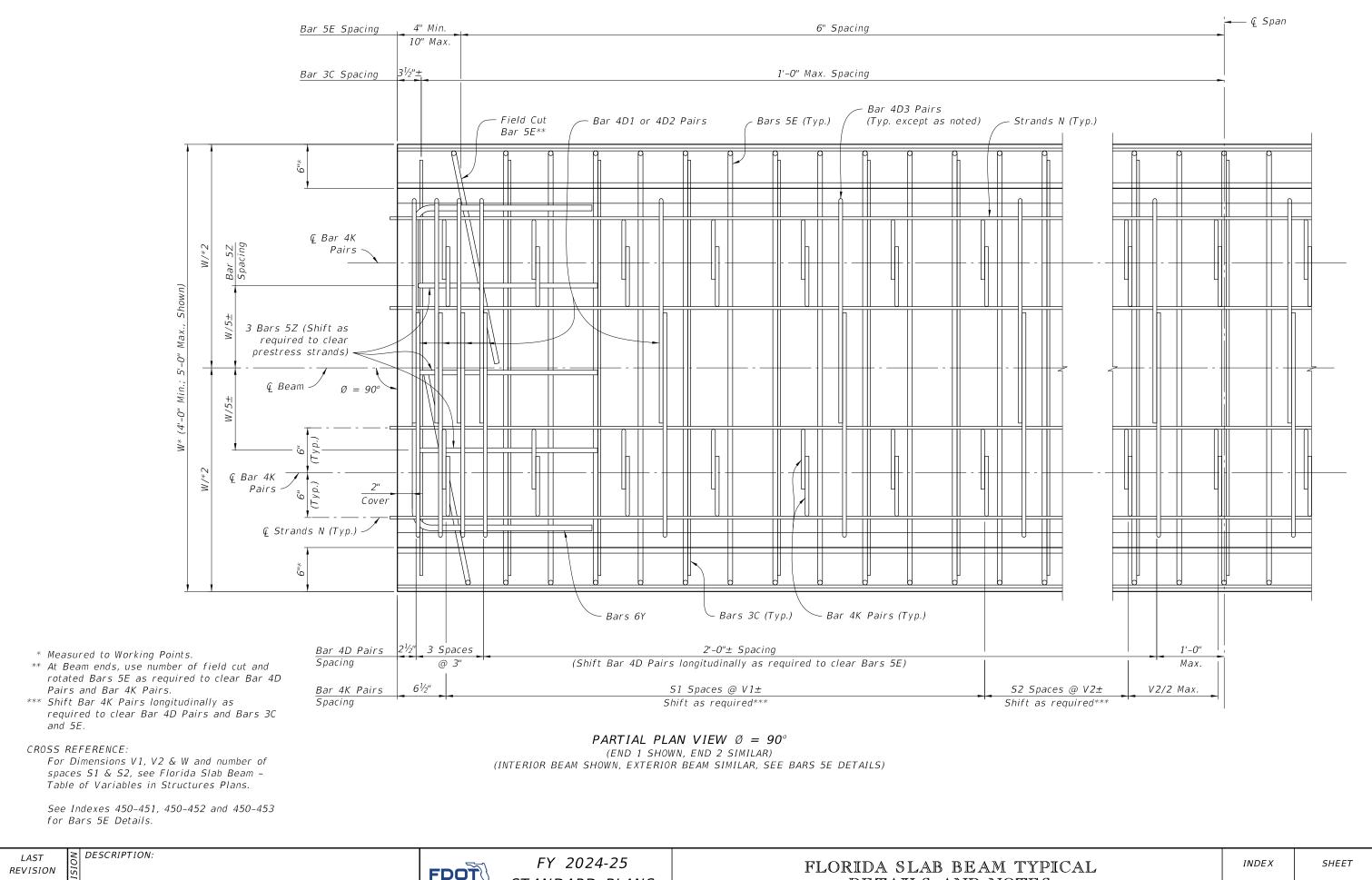


SCHEMATIC SIDE ELEVATION OF BEAM (Beam on a Positive Grade shown; Beam on a Negative Grade or Horizontal Grade similar.)



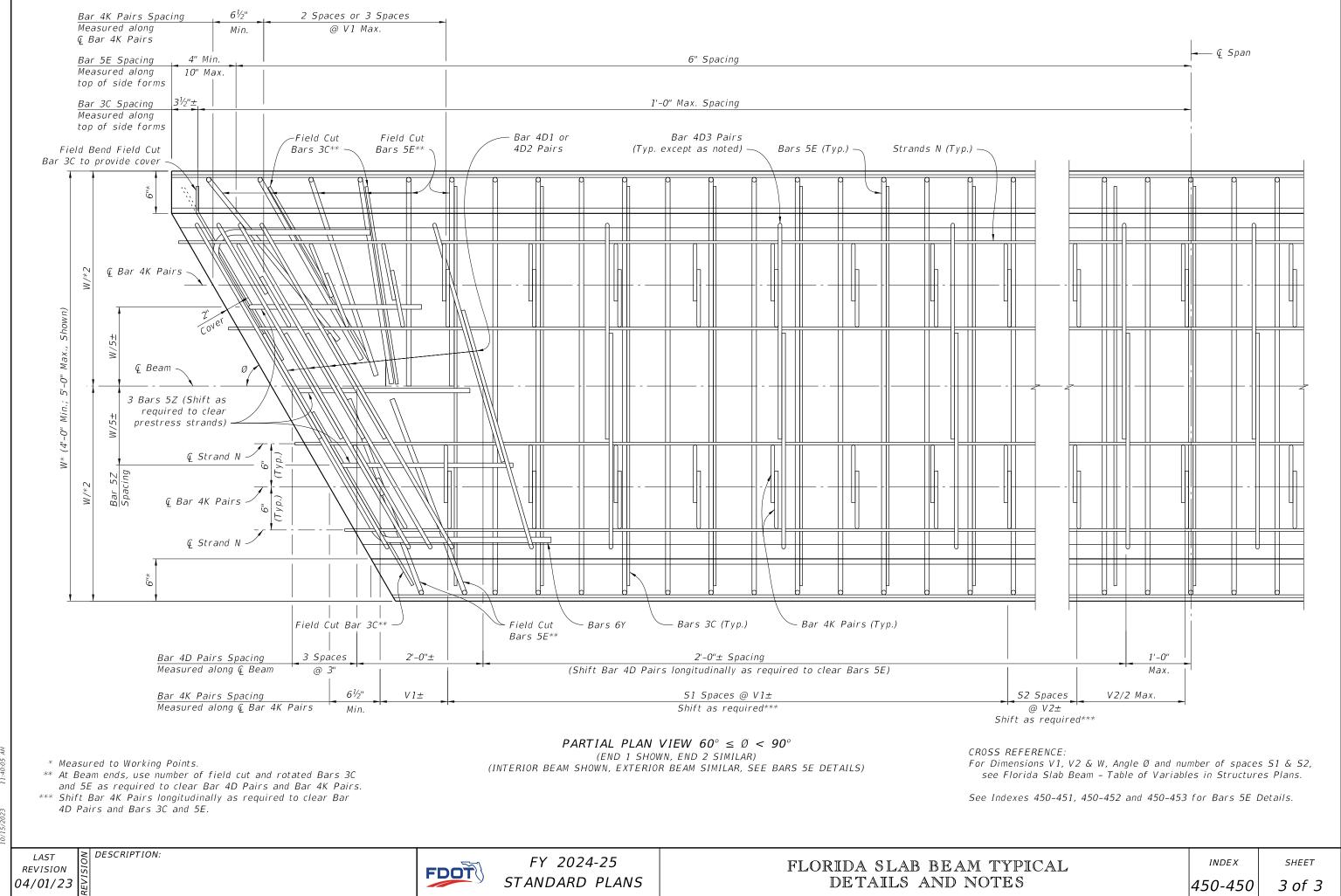
SCHEMATIC PLAN VIEWS AT BEAM ENDS

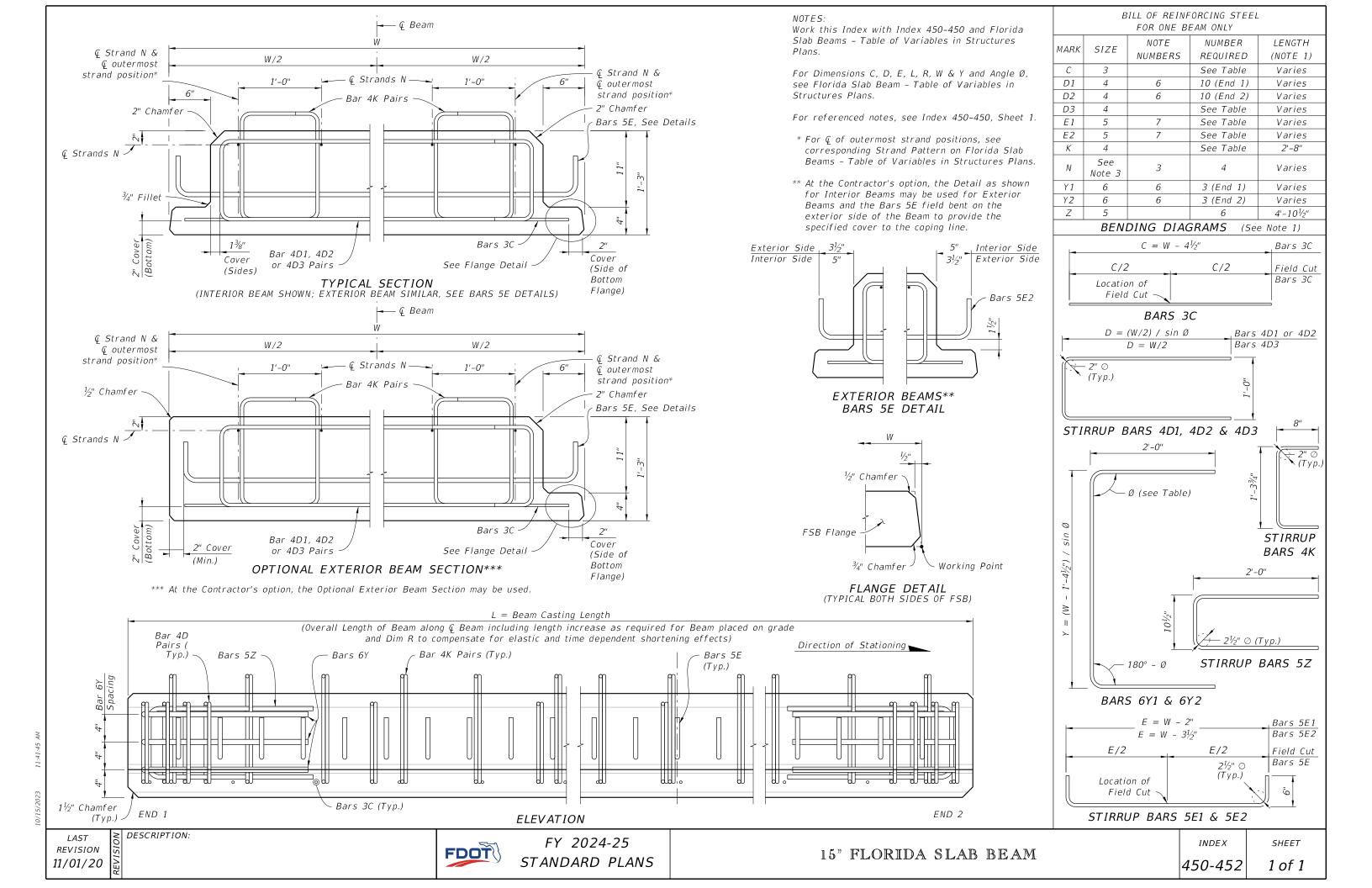
DESCRIPTION:



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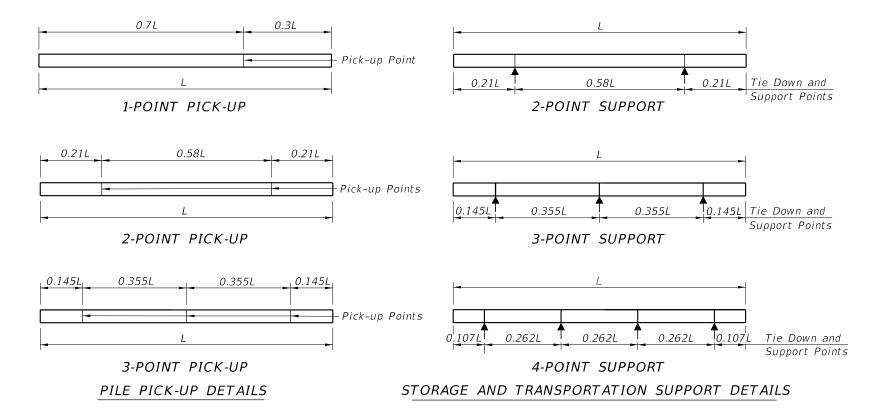
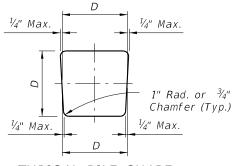
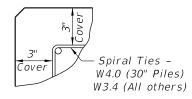


TABLE OF MAXIMUM PILE PICK-UP AND SUPPORT LENGTHS								
	D = Square Pile Size (inches)					Required Storage and	Diele IIn Detail	
	12	14	18	24	30	Transportation Detail	Pick-Up Detail	
Maximum Pile Length (Feet)	48	52	59	68	87	2, 3, or 4 point	1 Point	
	69	75	85	98	124	2, 3, or 4 point	2 Point	
	99	107	121	140	178	3 or 4 point	3 Point	



TYPICAL PILE SHAPE FOR MOLD FORMS



DETAIL SHOWING TYPICAL COVER

PRESTRESSED CONCRETE PILE NOTES:

- 1. Work this Index with the Square Prestressed Concrete Pile Splices (Index 455-002), the Prestressed Concrete Pile Standards (Index 455-012 thru 455-030), the High Moment Capacity Square Prestressed Concrete Pile (Index 455-031) and the Pile Data Table in the Structures Plans.
- 2. Concrete:
 - A. Piles: Class V, except use Class VI for High Moment Capacity Pile (Index 455-031).
 - 3. High Capacity Splice Collar: Class V.
 - C. See "GENERAL NOTES" in the Structures Plans for locations where the use of Highly Reactive Pozzolans is required.
- 3. Concrete strength at time of prestress transfer:
 - A. Piles: 4,000 psi minimum.
 - B. High Moment Capacity Piles: 6,500 psi minimum.
- 4. Carbon-Steel Reinforcing:
 - . Bars: Meet the requirements of Specification Section 415.
 - B. Prestressing Strands: Meet the requirements of Specification Section 933.
 - C. Protect all strands permanently exposed to the environment and not embedded under final conditions in accordance with Specification Section 450.
- 5. Spiral Ties:
 - A. Tie each wrap of the spiral strand to a minimum of two corner strands.
 - B. One full turn required for spiral splices.
- 6. Pile Splices: Fill dowel holes and form the joint between pile sections with a Type AB Epoxy Compound in accordance with Specification Section 926. Use an Epoxy Bonding Compound or an Epoxy Mortar as recommended by the Manufacturer.

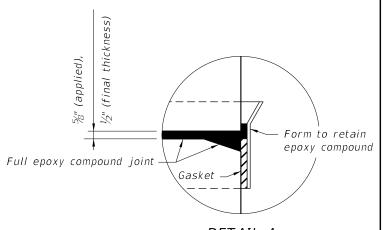
MA 51.745.13 AW

DESCRIPTION:

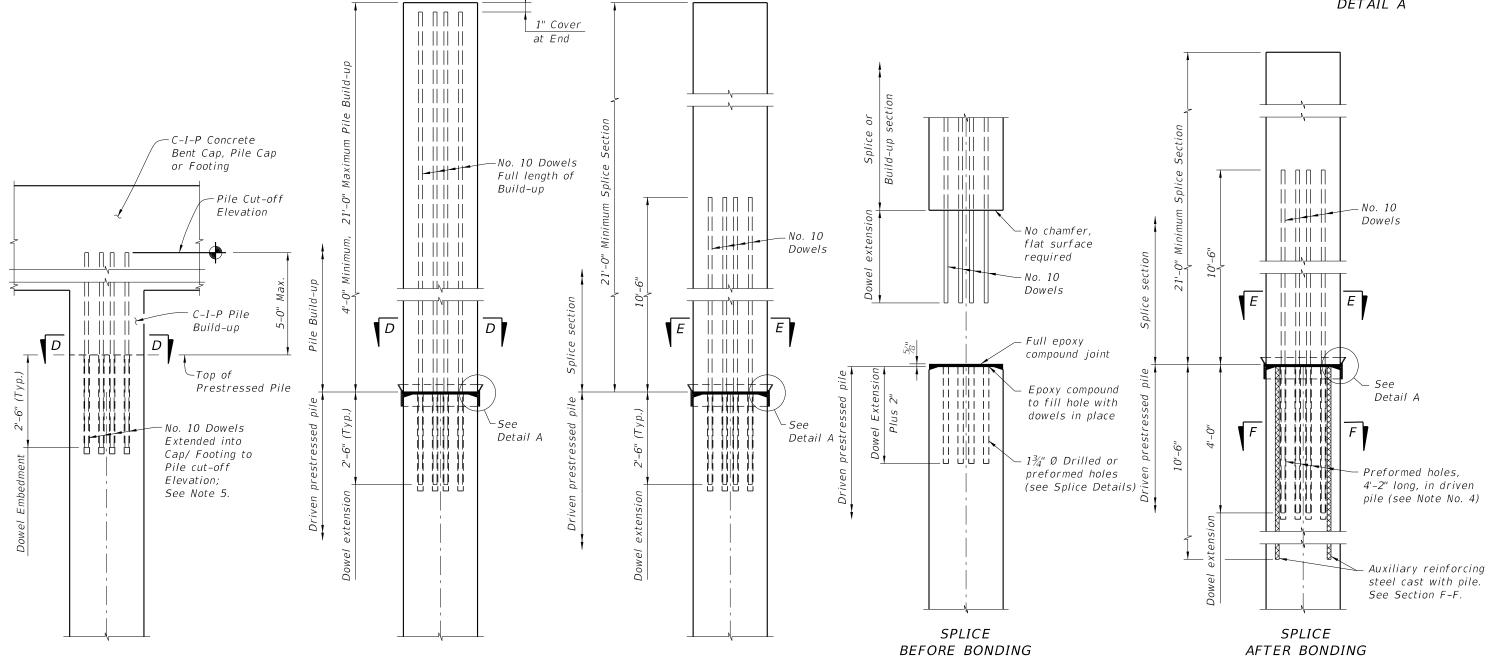
455-001 1 of 1

NOTES:

- 1. For Sections D-D, E-E, & F-F see Index 455-012 thru 455-030 for applicable concrete pile size and Pile Splice Reinforcement Details.
- 2. Prestressing strands, spiral ties and/or reinforcement are not shown for clarity.
- 3. When pile splices are necessary due to shipping and handling limitations, use the "Drivable Planned Prestressed Precast Splice Detail" or Mechanical Pile Splices on the Approved Products List (APL).
- 4. When preformed dowel holes are used, continue the 1" spiral tie pitch to 4'-0" below the head of the pile, See Index 455-018, 455-020 & 455-024. For preformed holes; use either removable preforming material or stay-in-place corrugated galvanized steel ducts meeting ASTM Specification A653, Coating Designation G90, 26 gauge. Use 2" diameter ducts with a minimum corrugation (rib) height of 0.12 in. fabricated with either welded or interlocked seams. Galvanizing of welded seams is not required.
- 5. For tension piles where top of Prestressed Pile is less than 3 feet below Pile Cut-off Elevation, extend No. 10 Dowels into cap beyond Pile Cut-off Elevation to achieve development as approved by the Engineer.



DETAIL A



DRIVABLE UNPLANNED

PRESTRESSED PRECAST

PILE SPLICE DETAIL

LAST

11/01/22

UNPLANNED

REINFORCED C-I-P

PILE BUILD-UP DETAIL

NON-DRIVABLE UNPLANNED

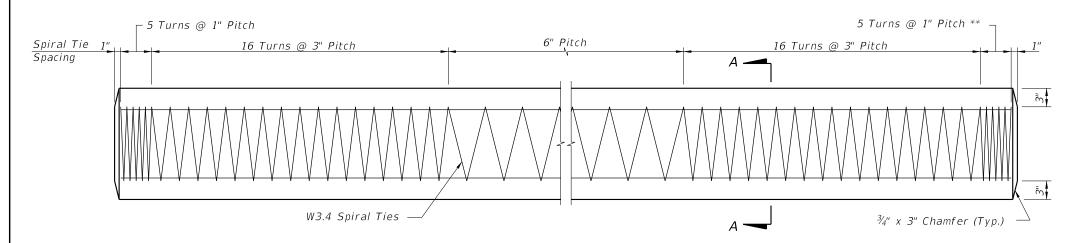
REINFORCED PRECAST

PILE BUILD-UP DETAIL

DRIVABLE PLANNED

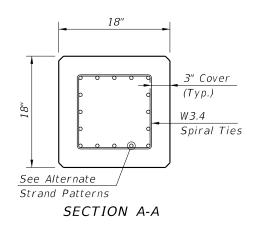
PRESTRESSED PRECAST

PILE SPLICE DETAIL



ELEVATION

** See Note 4 on Index 455-002



ALTERNATE STRAND PATTERNS

12 ~ 0.6" Ø, Grade 270 LRS, at 35 kips

 $12 \sim \frac{1}{2}$ " Ø (Special), Grade 270 LRS, at 34 kips

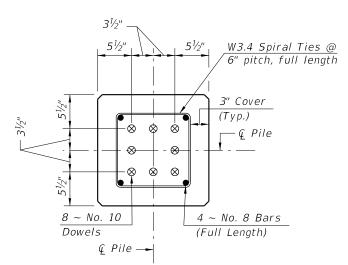
 $16 \sim \frac{1}{2}$ " Ø, Grade 270 LRS, at 26 kips

 $20 \sim \frac{7}{16}$ " Ø, Grade 270 LRS, at 21 kips

 $24 \sim \frac{3}{8}$ " Ø, Grade 270 LRS, at 17 kips

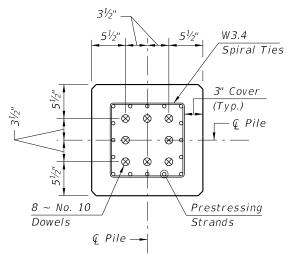
- 1. Work this Index with Index 455-001 Typical Details and Notes for Square Prestressed Concrete Piles and Index 455-002 - Square Prestressed Concrete Pile Splices.
- 2. Any of the given Alternate Strand Patterns may be utilized. The strands shall be located as follows:

Place one strand at each corner and place the remaining strands equally spaced between the corner strands. The total strand pattern shall be concentric with the nominal concrete section of the pile.



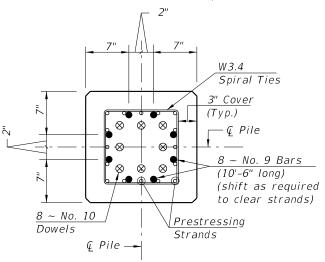
SECTION D-D

(See Non-Drivable Unforeseen Reinforced Precast Splice Detail)



SECTION E-E

(See Drivable Prestressed Precast Splice Detail)

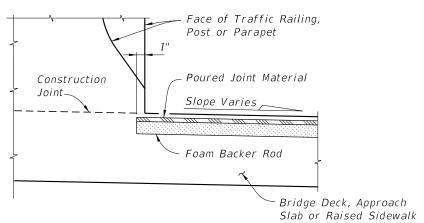


SECTION F-F

(See Drivable Preplanned Splice Detail)

PILE SPLICE REINFORCEMENT DETAILS

DESCRIPTION:



PARTIAL SECTION ALONG Q JOINT

JOINT TREATMENT AT HIGH SIDE OF

DECK WITH SLOPES 1% OR GREATER

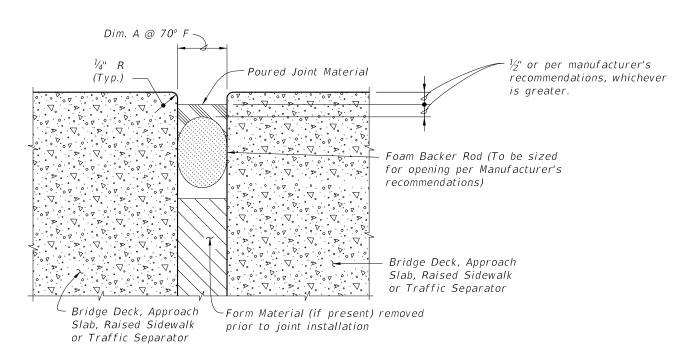
-Traffic Separator (Type and width vary, see Structures Plans) Poured Joint Material Angle Varies — Construction Joint Foam Backer Rod – Bridge Deck or Approach Slab

PARTIAL SECTION ALONG Q JOINT,

JOINT TREATMENT AT TRAFFIC SEPARATOR

Face of Traffic Railing Post or Parapet Approximate shape of Traffic Separator with Poured Joint Poured Joint Material Slope Varies - Upturn Construction Foam Backer Rod Joint Bridge Deck, Approach Slab or Raised Sidewalk-

PARTIAL SECTION ALONG Q JOINT JOINT TREATMENT AT LOW SIDE OF DECK OR HIGH SIDE OF DECK WITH SLOPES < 1%



GENERAL NOTES:

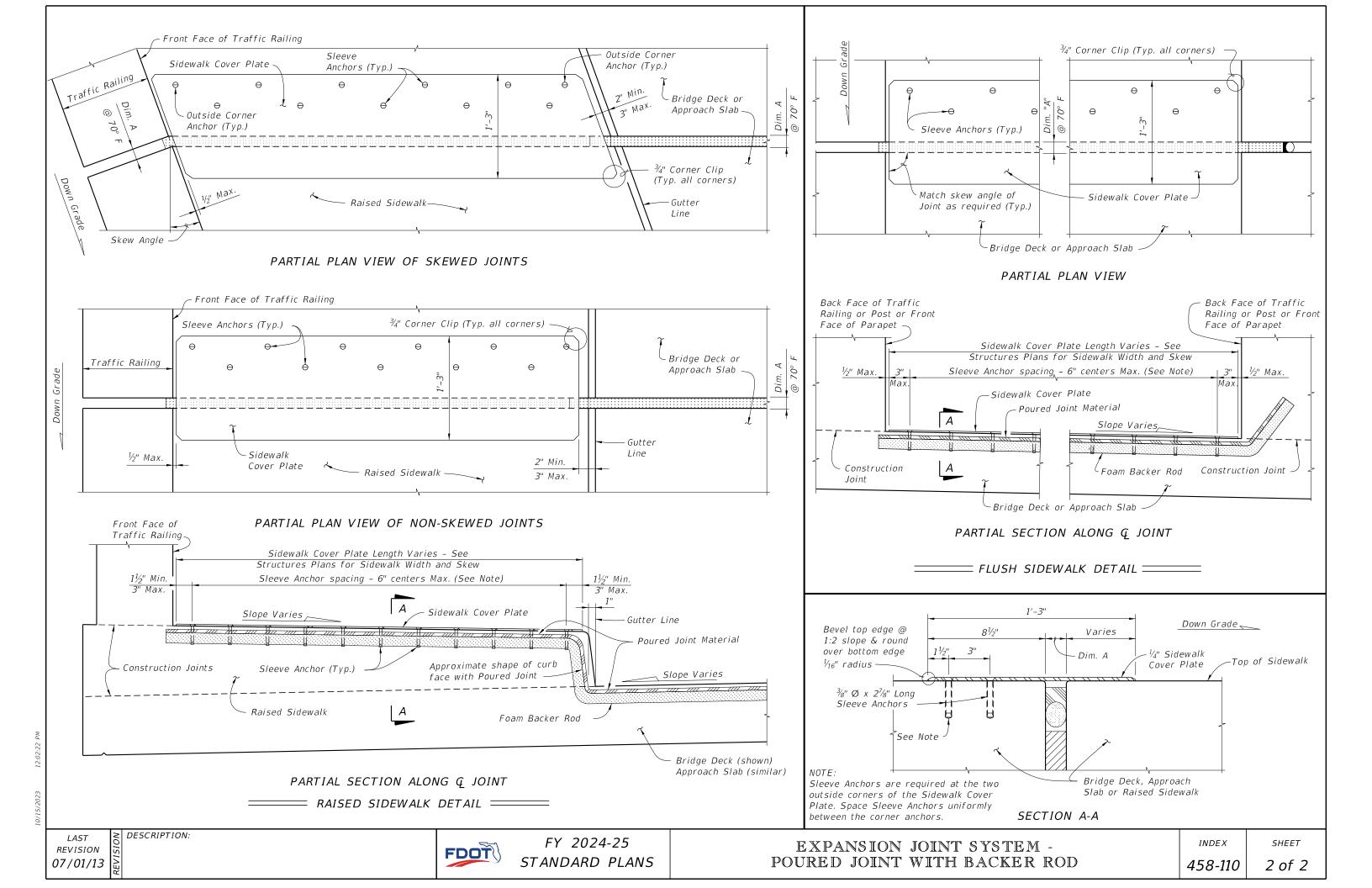
- 1. Furnish and install Poured Joint With Backer Rod Expansion Joint Systems in accordance with Specification Sections 458 and 932 using Type D silicone sealant material.
- 2. Refer to the Structures Plans, Poured Expansion Joint Data Table for Dim. A @ 70° F.

TYPICAL SECTION THRU JOINT

REVISION 11/01/23

DESCRIPTION:

FDOT



ELEVATION OF INSIDE FACE OF RAILING

(Scheme 2 shown with Post "A", other Schemes similar, Reinforcing Steel not shown for clarity)

for Pre-cured

Silicone Sealant

- 1. Shop Drawings are required.
- 2. Work this Index with Index 515-052 Bicycle/Pedestrian Railing Details (Steel) and Specification Section 515. Refer to the SPI for Design Criteria and Limits of Use.
- 3. Materials:
 - A. Steel: Galvanized after fabrication
 - a. Fasteners: Hex Head Bolt ASTM A307, Hex Nuts ASTM A563, Washers ASTM F436
 - b. Support Bracket (Scheme 3) L-shape and Stiffener Plate: ASTM A36
 - c. Bottle-guard (Schemes 1 & 3) L-shape: ASTM A36
 - B. Concrete: Same as bridge deck
 - C. Pre-cured Silicone Sealant: Specification Section 932
 - D. Bearing Pads: Provide \(\frac{1}{18} \)" Plain, Fabric Reinforced or Fabric Laminated bearing pads that meet the requirements of Specification Section 932 for Ancillary Structures.
- 4. See Structures Plans, Superstructure Sheets for bridge information including concrete type, deck expansion joint locations and orientations, and thermal movement.
- 5. Railings:
 - A. For thermal movement greater than 4" (up to a maximum of 5"), clear opening between adjacent pickets, or panels at Rail Expansion Joints above Deck Joints must be reduced to $3\frac{1}{3}$ ".
 - B. For treatment of railings on skewed bridges see Index 521-427.
- 6. Curbs:
 - A. Match open curb joints at Deck Expansion Joint locations to the deck joint dimension.
 - B. Construct Concrete Curb (Scheme 2) vertical with the top surface finished level transversely. See Concrete Curb Details Sheet 3.
 - C. Provide $\frac{3}{4}$ " Intermediate open joints in curbs coinciding with the $\frac{3}{4}$ " joints in the traffic railing.
- 7. Payment: Support bracket (Scheme 3) is incidental to the cost of railing. Curb concrete and reinforcing steel (Scheme 2) are included in the bridge deck quantities.

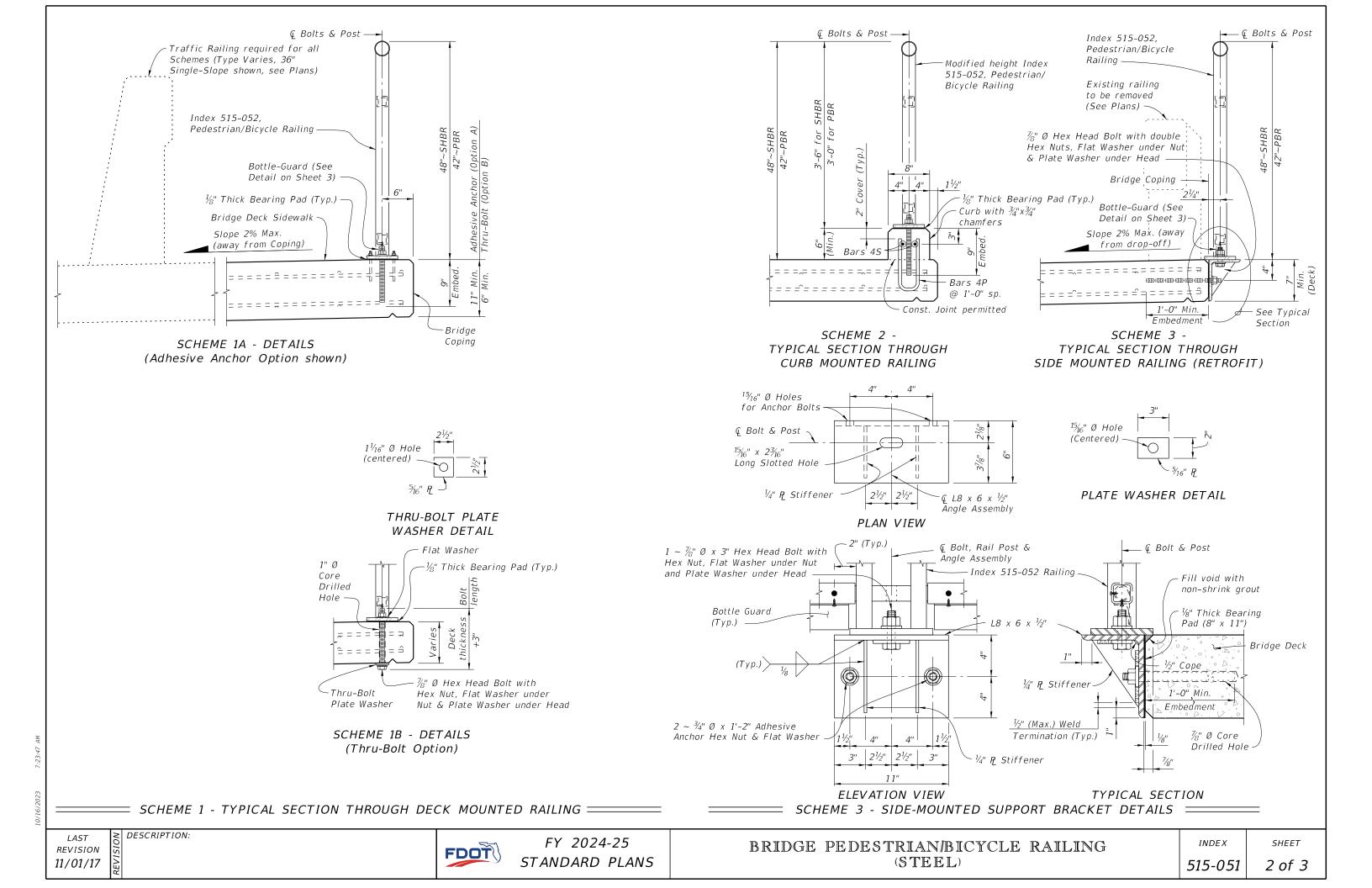
* Deck Joint at Begin Bridge or End Bridge shown; Deck Joint at Q Pier or Intermediate Bent similar.

** SHBR~Special Height Bicvcle Rail PBR~Pedestrian/Bicycle Rail

REVISION

11/01/17

DESCRIPTION:

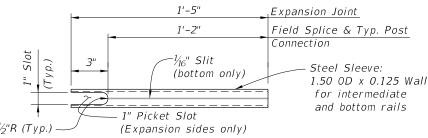


ROUND RAILS - TOP RAIL OR HANDRAIL

- * ¼" Ø x ¾" Pan Head Stainless Steel (Type 316 or 18-8 Alloy) Set Screws along outside face of railing. Set screws must be set flush against the rail surface. A ¾" Ø plug weld may be substituted for the two set screws at expansion joints.
- ** Embedded length may be 4" for plug welded connection.
 *** Increase handrail sleeve embedment to 8" for Expansion Joint openings
- greater than 2".

 **** Expansion Joint opening shall match the clear opening in the deck joint but not greater than 3".

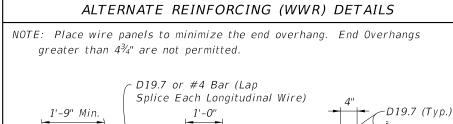
sharp edges (Typ.)

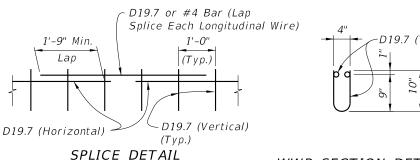


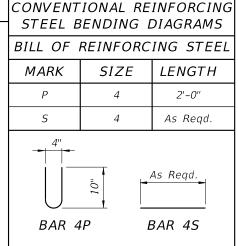
INTERMEDIATE OR BOTTOM RAIL - STEEL SLEEVE DETAIL (Bottom Side Shown)

 \equiv DETAIL "B" EXPANSION JOINT (FIELD SPLICE SIMILAR) =

WWR SECTION DETAIL







CURB REINFORCING STEEL NOTES:

(Between WWR Sections)

ESTIMATED CONCRETE CURB

QUANTITIES (SCHEME 2)

UNIT

CY/LF

LB/LF

ITEM

Reinforcing Steel

Concrete

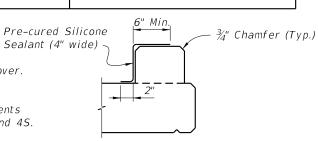
DESCRIPTION:

- 1. All bar dimensions in the bending diagrams are out to out.
- 2. The reinforcement for the curb on a retaining wall shall be the same as detailed for an 8" deck.
- 3. All reinforcing steel at the open joints shall have a 2" minimum cover.
- 4. Bars 4S may be continuous or spliced at the construction joints. Bar splices for Bars 4S shall be a minimum of 1'-8".
- 5. Deformed Welded Wire Reinforcement (WWR) meeting the requirements of Specification Section 931 may be used in lieu of all Bars 4P and 4S.

QUANTITY

0.0124

4.01

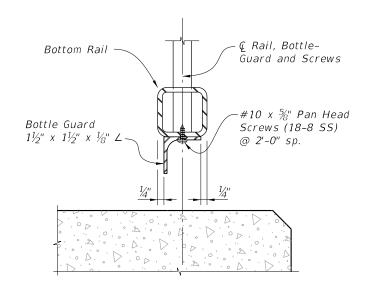


DETAIL "A" - SECTION

AT INTERMEDIATE OPEN JOINT INTERMEDIATE JOINT SEAL NOTE:

At Intermediate Open Joints, seal the lower 6" portion of the open joint with Pre-cured Silicone Sealant. Apply sealant prior to any Class V finish coating and remove all curing compound and loose material from the surface prior to application of bonding agent.

SCHEME 2 - CONCRETE CURB DETAILS

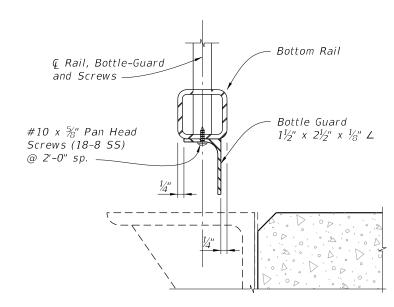


TYPICAL SECTION THROUGH BOTTOM RAIL (Post Not Shown for Clarity)

== SCHEME 1 - BOTTLE GUARD DETAIL ====

CROSS REFERENCE:

See Sheet 1 for Bridge Railing Notes.



TYPICAL SECTION THROUGH BOTTOM RAIL (Post Not Shown for Clarity)

= SCHEME 3 - BOTTLE GUARD DETAIL =

LAST REVISION 11/01/16

FDOT

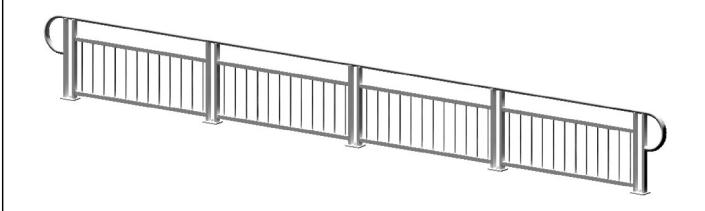
FY 2024-25 STANDARD PLANS

BRIDGE PEDESTRIAN/BICYCLE RAILING (STEEL)

INDEX

SHEET

515-051 3 of 3



3D VIEW OF RAILING WITH TYPE 1 - PICKET INFILL PANEL (42" Height shown, 48" Height Similar)

TABLE 1 - RAILING MEMBERS					
MEMBER	DESIGNATION	OUTSIDE DIMENSION	WALL THICKNESS		
Post "A"	HSS 2½ x 1½ x½	2.50" x 1.50"	0.125"		
Post "B"	HSS 2½ x 1½ x¾ ₁₆	2.50" x 1.50"	0.188"		
Tan Bail	2½" NPS (Sch. 10)	2.875"	0.120"		
Top Rail	HSS 3.000 x 0.120	3.000"	0.120"		
Fad Uses	2½" NPS (Sch. 10)	2.875"	0.120"		
End Hoops	HSS 3.000 x 0.120	3.000"	0.120"		
Top Rail Joint/Splice Sleeves	HSS 2.500 x 0.125	2.500"	0.125"		
Intermediate & Bottom Rail	HSS 2 x 2 x $\frac{3}{16}$	2.00" x 2.00"	0.188" ⁽¹⁾		
Int. & Bottom Rail Post Connection Sleeve	HSS 1.500 x 0.125	1.500"	0.125" ⁽¹⁾		
Handrail Joint (Calica Classes	1" NPS (Sch. 40)	1.315"	0.133"		
Handrail Joint/Splice Sleeves	HSS 1.500 x 0.125	1.500"	0.125"		
Handrails	1½" NPS (Sch. 40)	1.900"	0.145"		
Handrail Support Bar	¾" Ø Round Bar	0.750"	N/A		
Pickets (Type 1 Infill Panel)	¾" Ø Round Bar	0.750"	N/A		
Infill Panel Members (Types 2 - 5)	Varies (See Details)	Varies	Varies		

TABLE 1 NOTES:

(1) 0.125" wall thickness permitted for rails with post spacings less than 5'-8", except that Post Connection Sleeve must be 1\frac{1}{4}" NPS (Sch. 40).

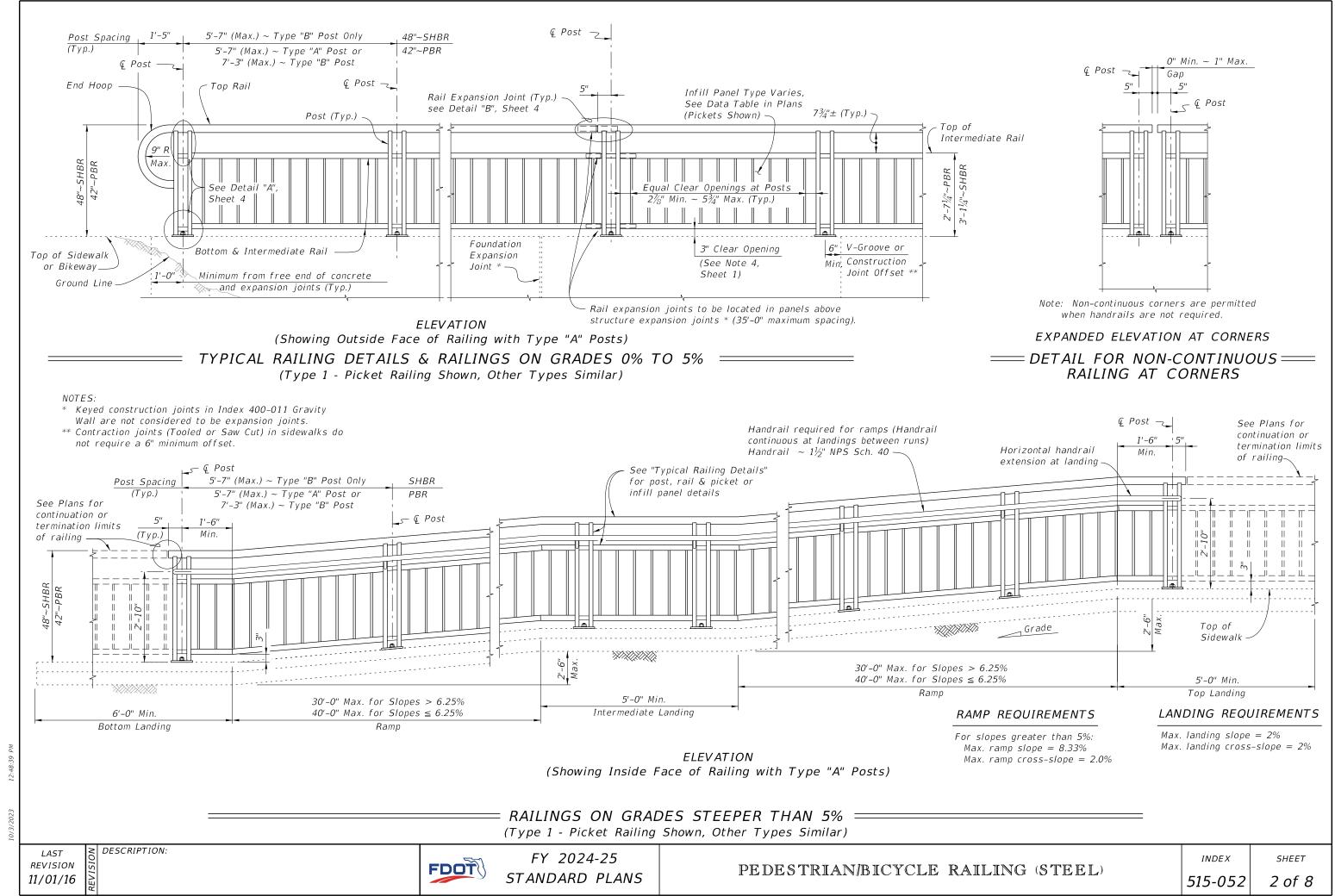
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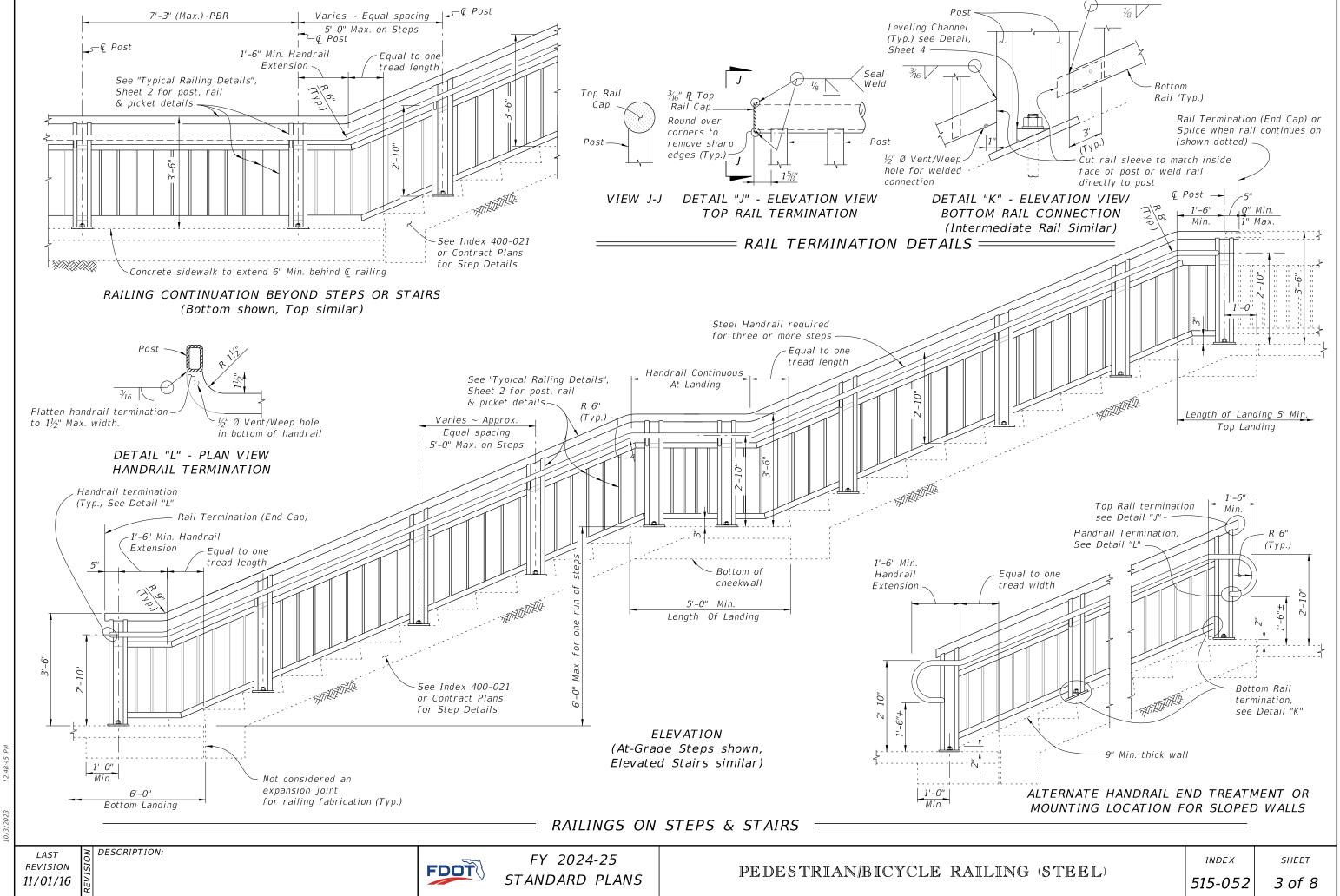
Notes:

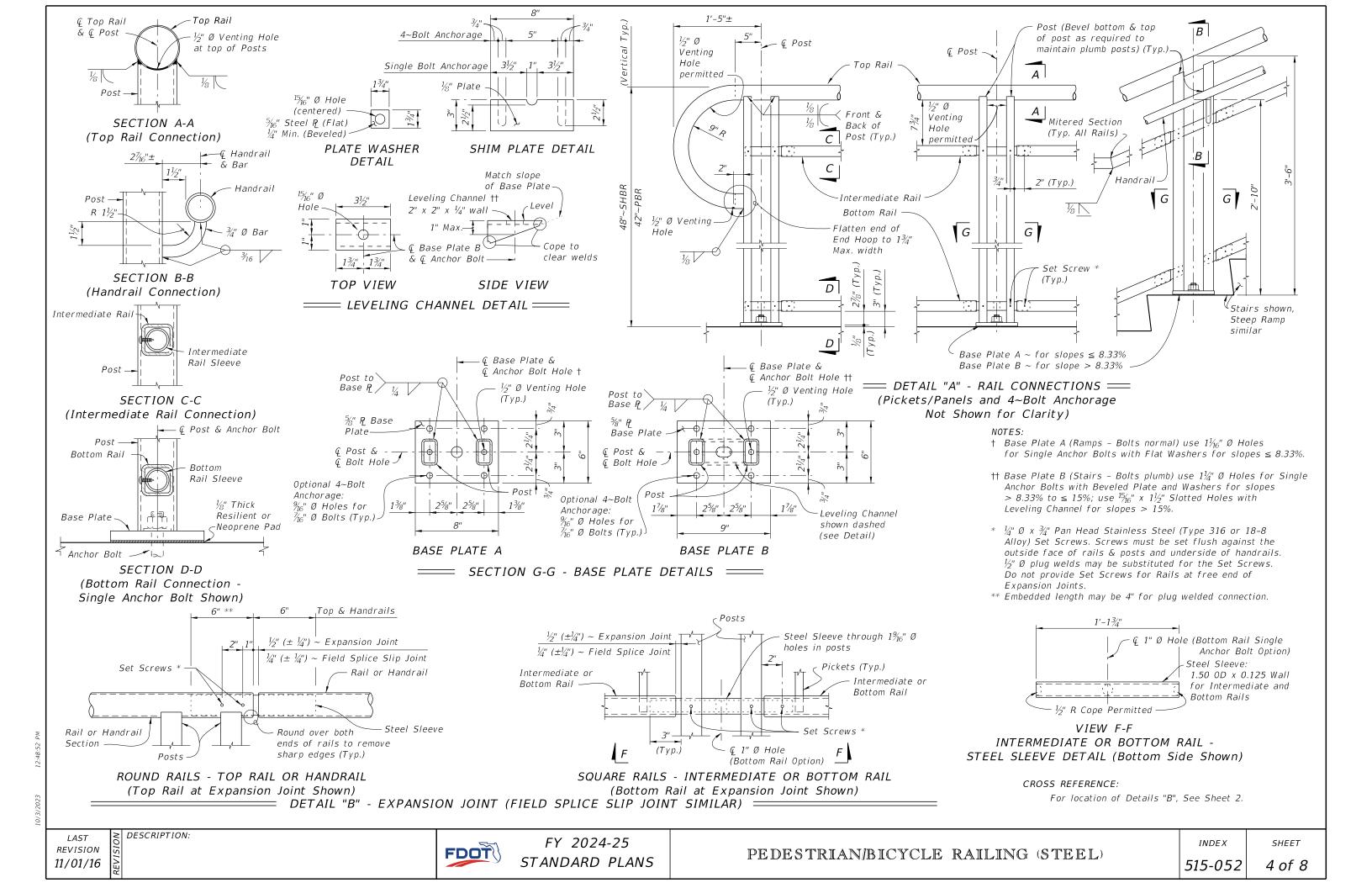
- 1. Shop Drawings are required; see Specification Section 515
- 2. For bridge mounted railings work this Index with Index 515-051 Bridge Bicycle/Pedestrian Railing
- 3. Materials:
 - A. Pipe Rails and Pickets: ASTM A500 Grade B, C or D, or ASTM A53 Grade B for standard weight pipe (Schedule 40) and ASTM A36 for bars.
 - B. Structural Tube: ASTM A500 Grade A, B, C, or D or ASTM A501
 - C. Steel Plate: ASTM A36 or ASTM A709 Grade 36
 - D. U-Channels and filler plates: ASTM A36 or ASTM A1011 (Grade 36).
 - E. Stainless steel (SS) screws: Type 316 or 18-8 Alloy
 - F. Galvanized Steel Fasteners: coated in accordance with Specification Section 962.
 - a. Hex Head Bolts: ASTM A 307
 - 1. $\frac{1}{8}$ " diameter single bolt option, Grade 36
 - 2. $\frac{7}{16}$ " four bolt option, Grade 55
 - b. Adhesive Anchors: ASTM F1554 fully threaded rods, Grade 55
 - c. Hex Nuts: ASTM A563
 - d. Flat Washers: ASTM F436
 - e. Plate Washers: ASTM A36 or ASTM A706 Grade 36.
 - G. Shims: ASTM B209 Alloy 6061
 - H. Bearing Pads: $lac{1}{2}$ " Plain, Fabric Reinforced or Fabric Laminated pads that meet the
 - requirements of Specification Section 932 for Ancillary Structures.
- 4. Fabricate pickets and vertical panel elements parallel to the posts; except Type 2, 3 and 5 panel infills may be fabricated parallel to the longitudinal grade. Maintain a maximum clear opening of 5% for standard installations and 3% when a 4" sphere requirement is indicated in the Data Tables.
- 5. Maximum spacing between expansion joints is 40'-0". Locate an Expansion Joint between the posts on either side of the Deck Expansion Joint.
- 6. Field splices are similar to the Expansion Joint Detail and may be approved by the Engineer to facilitate handling; but the top rail must be continuous across a minimum of two posts.
- 7. For intermediate and bottom horizontal rails, the screwed joints shown may be substituted with alternate joints shown in detail "K".
- 8. Make corners and changes in tangential longitudinal alignment with a 9" bend radius or terminate adjoining sections with mitered end sections when handrails are not required.
- 9. For changes in tangential longitudinal alignment greater than 45°, position posts a maximum of 2'-0" each side of the corner but not at the corner apex.
- 10. For curved longitudinal alignments, shop bend the top and bottom rails and handrails to match the alignment radius.
- 11. Handrails are required and must be continuous at landings for:
 - A. Grades Steeper than 5%,
 - B. Three or more steps
- 12. Installation: Cutting of reinforcing steel is permitted for post installed anchors.

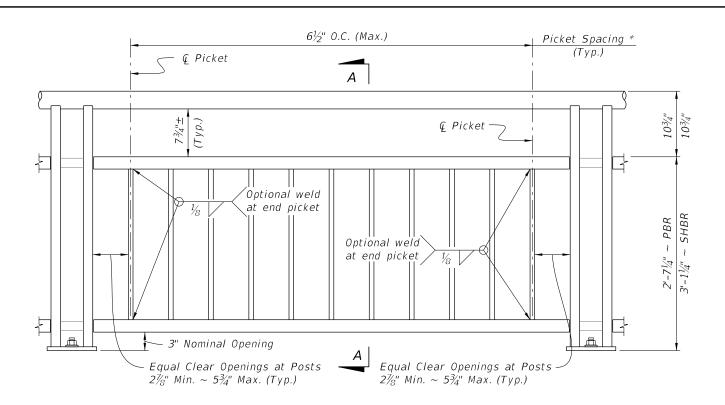
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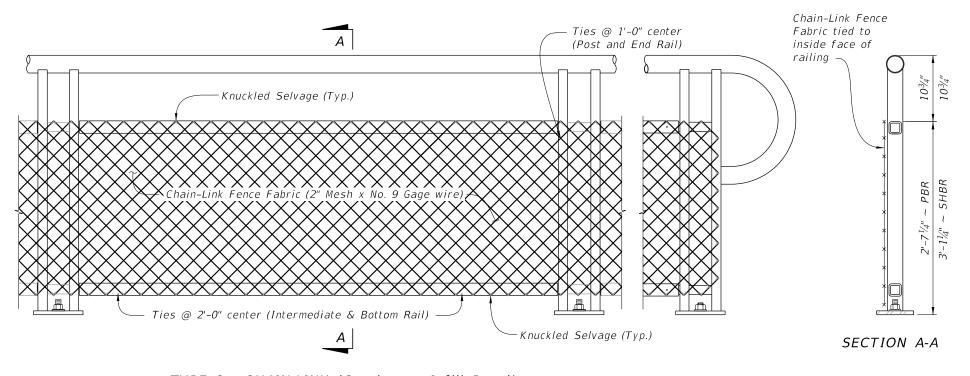






PICKET NOTES:

* Picket Spacing of $6\frac{1}{2}$ " centers is based on a $\frac{3}{4}$ " Ø Bar for standard applications. When shown in the Contract Plans a $4\frac{1}{2}$ " picket spacing may be required. See Note 4 (Sheet 1).



TYPE 2 - CHAIN-LINK (Continuous Infill Panel)

NOTES:

DESCRIPTION:

1. See Plans for Infill Panel option required.

TABLE 2 - CHAIN-LINK PANEL COMPONENT MATERIALS					
COMPONENT	ASTM	COMPONENT INFORMATION			
Chain-Link Fence Fabric (2" mesh with	A 392	Zinc-Coated Steel - No. 9 gage (coated wire diameter), Class 2 Coating			
knuckled top and bottom selvage)	A 491	Aluminum-Coated Steel - No. 9 gage (coated wire diameter)			
	F 668	Polyvinyl Chloride (PVC) Coated Steel - No 9 gage Zinc-Coated Wire (metallic-coated core wire diameter) ~ See Plans for specified color of PVC.			
Tie Wires	F 626	Zinc-Coated Steel Wire - No. 9 gage wi coating to match Chain-Link Fence Fabr			
Tension Bars	F 626	$^{3}\!\!/_{16}$ " (Min. thickness) x $^{3}\!\!/_{4}$ " (Min. width) x 2'-3' (Min. height) Steel Bars			
Miscellaneous Fence Components	F 626	Zinc-Coated Steel			

- ¾" Max.

DETAIL "1A"
(Top of Picket Connection)

DETAIL "1B"

(Bottom of Picket Connection)

(Shown dashed) Picket ~ ¾" Ø Bar (Typ.)

Picket ~ ¾" Ø Bar (Typ.)

¹¾″ Ø Max. Hole for Ramps,

15/16" Ø Max. Hole for Stairs.

(Optional weld at end picket)

½" Thick Resilient or Neoprene Pad

45° Beveled End Permitted

CHAIN-LINK PANEL NOTE:

Intermediate Rail

Bottom Rail

45° Beveled

End Permitted

(Shown dashed)

Base Plate —

√ Anchor Bolt

¹³/₁₆" Ø Max. Hole for Ramps, ¹⁵/₁₆" Ø Max. Hole for Stairs.

(Optional weld at end picket)

See Detail "1A"

See Detail "1B"

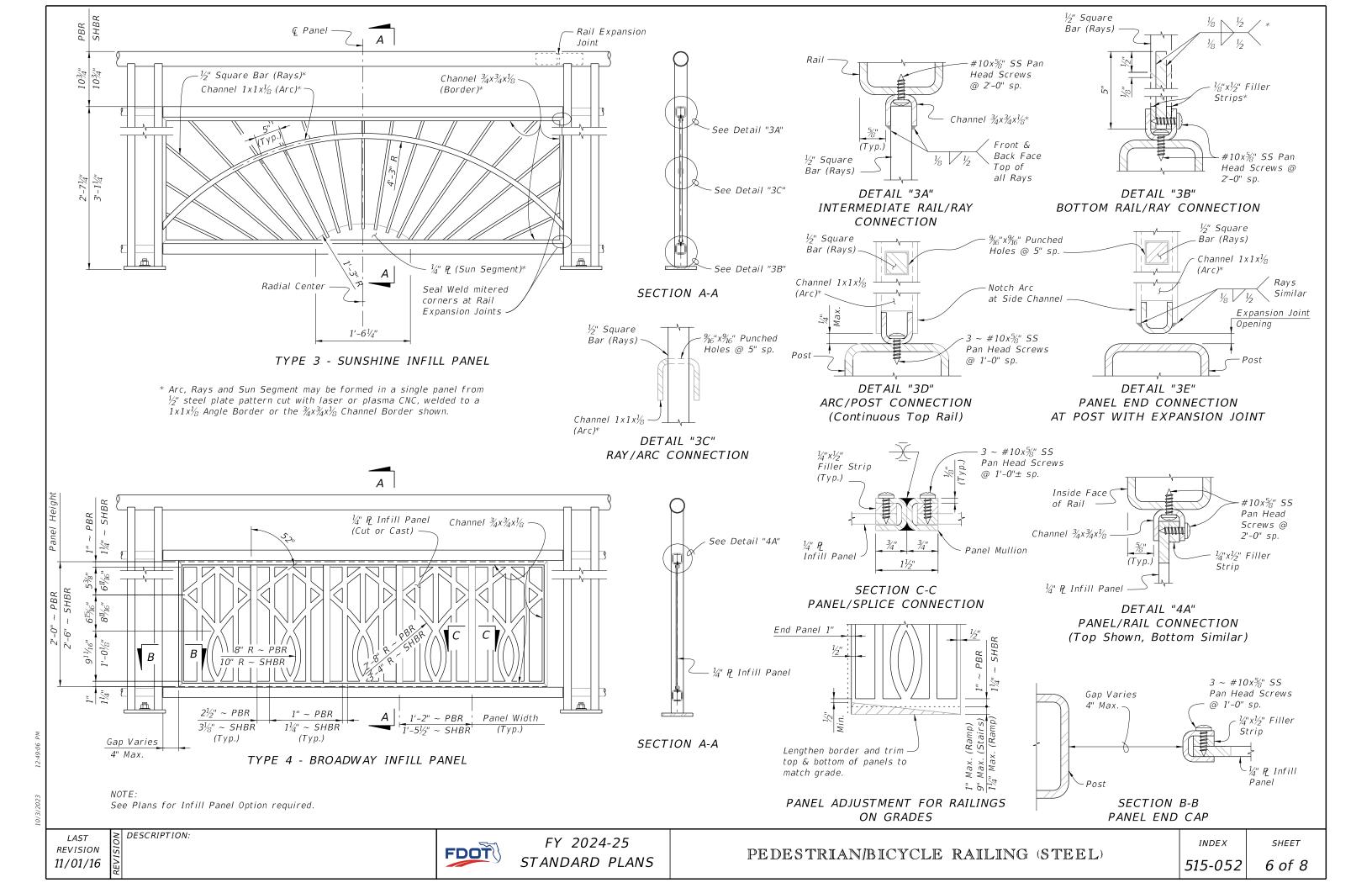
SECTION A-A

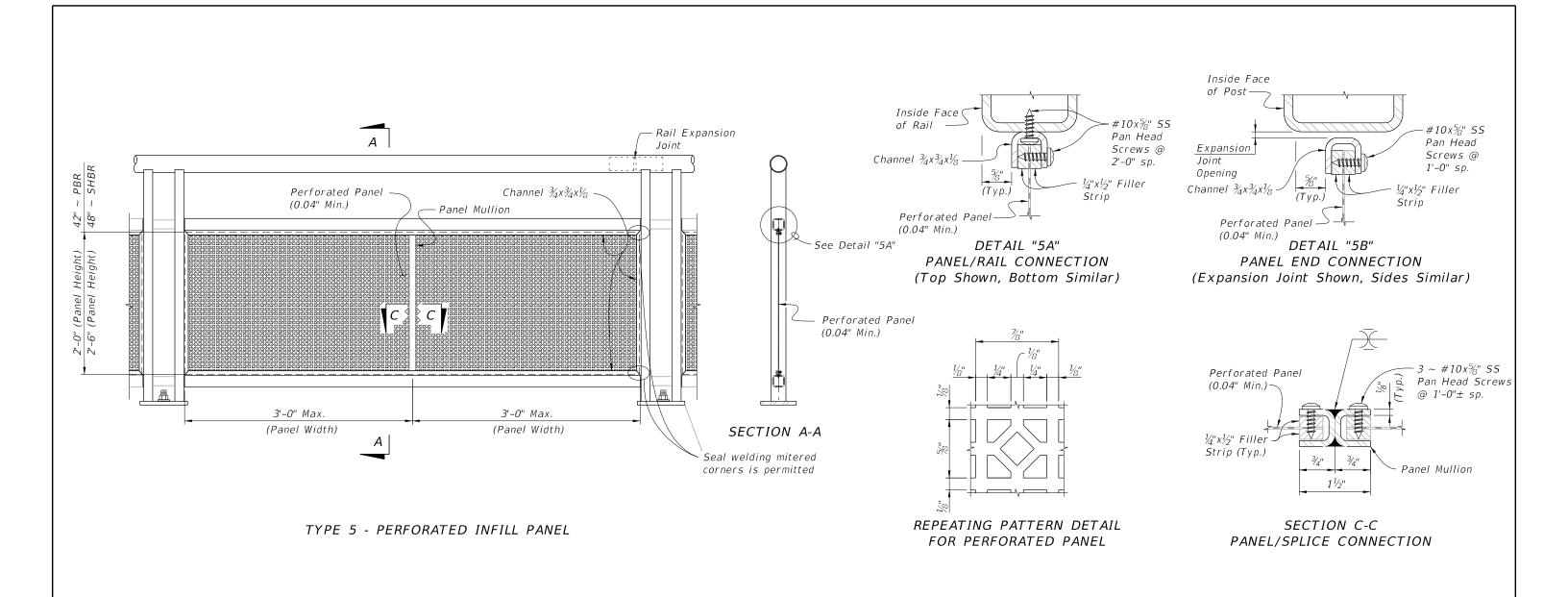
Chain-Link Fence Fabric shall be continuous along limits of railing. Splicing of Chain-Link panels using Tension Bars at 20'-0" minimum increments is permitted.

LAST REVISION 11/01/21



10/3/2023



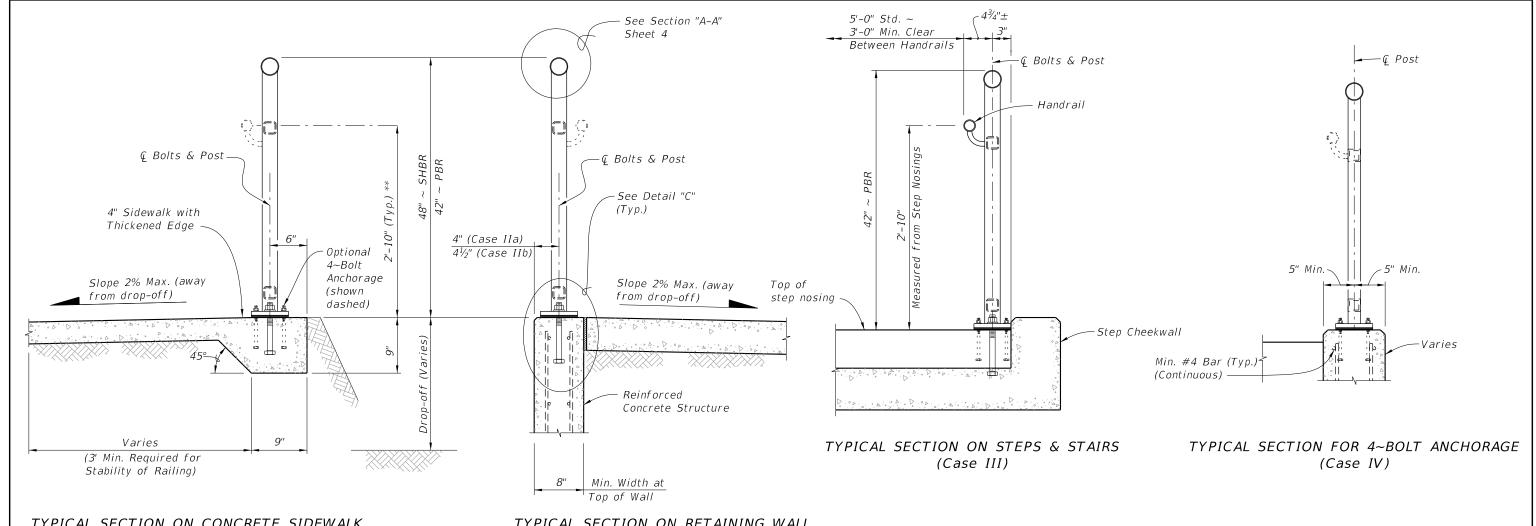


NOTES:

DESCRIPTION:

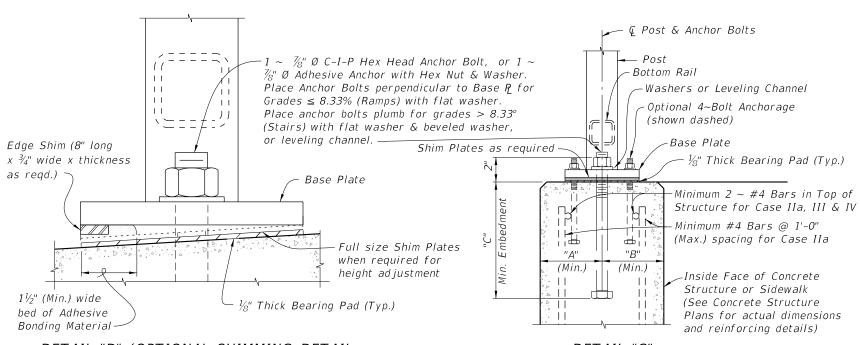
1. See Plans for Infill Panel Type required.

REVISION 11/01/16



TYPICAL SECTION ON CONCRETE SIDEWALK (Case I)

TYPICAL SECTION ON RETAINING WALL (Case II)



ANCHOR BOLT TABLE							
CASE	CTRUCTURE	D	IMENSIONS		ANCHOR LENGTH		ANGUOR
	STRUCTURE TYPE	A Edge Dist.	B Edge Dist.	C Embedment	C-I-P Hex Head Bolt		ANCHOR SIZE
I	Unreinforced Concrete	6"	1'-2"	6"	7½"	8"	%" Ø
IIa	Reinforced Concrete	4"	4"	9"	10½"	1 1"	%" Ø
IIb	Gravity Wall Index 400-011	41/2"	3 ¹ / ₂ " @ top	9"	10½"	1 1"	7⁄8" Ø
III	Step Cheekwall	4 ¹ / ₂ "	4 ¹ /2"	9"	10 ¹ /2"	1 1"	%" Ø
IV	Varies	5"	<i>5</i> "	5"	$6\frac{1}{2}$ "	7"	7∕16" Ø

** When required; measured from top of sidewalk.

DETAIL "D" (OPTIONAL SHIMMING DETAIL FOR CROSS SLOPE CORRECTION) (Used in lieu of Beveled Shim Plates)

DESCRIPTION:

DETAIL "C" (Cast-In-Place Anchor Bolts shown, Adhesive Anchors similar)

REVISION 11/01/20

FDOT

FY 2024-25 STANDARD PLANS

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PEDESTRIAN/BICYCLE RAILING (STEEL)